STRUCTURAL NOTES:

<u>GENERAL</u>

THE STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH THE DRAWINGS OF ALL OTHER DISCIPLINES AND THE SPECIFICATIONS. THE CONTRACTOR SHALL VERIFY THE REQUIREMENTS OF OTHER TRADES AS TO INSERTS, ANCHORS, SLEEVES, AND OTHER ITEMS TO BE PLACED OR SET IN THE STRUCTURAL WORK.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR COMPLYING WITH ALL SAFETY PRECAUTIONS AND REGULATIONS DURING THE WORK. THE CONTRACTOR SHALL BE RESPONSIBLE TO CONFIRM ALL THE FIELD DIMENSIONS ANY UNUSUAL CONSTRUCTION CONDITION THAT JEOPARDIZE SAFETY OF LABOR AND/OR PUBLIC, CONTRACTOR SHALL CONTACT ENGINEER IMMEDIATELY BEFORE PROGRESS, IN CASE OF AT THE TIME AND/OR IN FUTURE OR IN CASE OF THE ISCREPANCIES IN THE PROJECT

STRUCTURAL DESIGN SPECIFICATIONS

BUILDING CODE: 2018 INTERNATIONAL BUILDING CODE AS APPLICABLE 100 MPH PER ASCE 7-16 WIND MAP WIND VELOCITY (STRENGTH LEVEL) SNOW LOAD: 80 PSF WIND EXPOSURE CATEGORY: SEISMIC DESIGN CATEGORY: ROOF: - DEAD LOAD: 15 PSF FLOOR: - LIVE LOAD: 40 PSF

2000 PSF

SOIL:

- DEAD LOAD: 15 PSF

SCOPE: ANY ITEMS NOT SPECIFIED HEREIN SHALL FOLLOW THE REQUIREMENTS OF THE INTERNATIONAL RESIDENTIAL CODE'S PRESCRIPTIVE REQUIREMENTS. SUCH ITEMS MAY INCLUDE DETAILING OF FRAMING CONNECTIONS. SIZES OF MEMBERS, MATERIAL SPECIFICATIONS, AND OTHER REQUIREMENTS RELATED TO THE STRUCTURE. WHERE MANUFACTURED PRODUCTS ARE USED, THE DETAILING AS ESTABLISHED BY THE MANUFACTURER SHALL BE USED. THESE SPECIFICATIONS ARE BASED UPON CALCULATIONS FOR THE PROJECT. THE CALCULATIONS UTILIZE THE DOCUMENTS LISTED, AND ALL OF THE REFERENCED STANDARDS.

FRAMING NOTES

PROVIDE DOUBLE JOISTS UNDER ALL NON-LOAD BEARING PARTITIONS PARALLEL TO THE SPAN OF THE FLOOR JOISTS.

PROVIDE DIAGONAL OR SOLID BLOCKING @ 8'-0" O.C. MAXIMUM IN ALL FLOOR JOISTS AND SOLID BLOCKING BETWEEN FLOOR JOISTS UNDER WALLS THAT ARE PERPENDICULAR TO THE FLOOR JOISTS.

ALL STRUCTURAL WOOD USED IN THE CONSTRUCTION OF STEPS, PORCHES & DECKS, OR EXPOSED TO WEATHER SHALL BE PRESSURE TREATED (U.N.O).

ALL WOOD IN CONTACT W/ CONCRETE SHALL BE PRESSURE TREATED.

CAST-IN-PLACE CONCRETE NOTES

CONCRETE MIXES SHALL BE DESIGNED PER ACI 301. USING PORTLAND CEMENT CONFORMING TO ASTM C-150 OR C-595. AGGREGATE CONFORMING TO ASTM C-33, AND ADMIXTURES CONFORMING TO ASTM C-494, C-1017, C818, AND C-260, CONCRETE SHALL BE READY MIXED IN ACCORDANCE WITH ASTM C-94.

CONCRETE SHALL CONFORM TO THE FOLLOWING: LOCATION MIN f'c

FOUNDATION 3,500 PSI SLAB ON GRADE 4,000 PSI

REINFORCING STEEL, INCLUDING HOOKS AND BENDS, SHALL BE DETAILED IN ACCORDANCE WITH ACI 315. ALL REINFORCING STEEL INDICATED AS BEING CONTINUOUS SHALL BE LAPPED WITH A TYPE 2 SPLICE UNLESS OTHERWISE NOTED.

BAR SUPPORTS SHALL BE PROVIDED FOR ALL REINFORCING STEEL TO ENSURE MINIMUM CONCRETE COVER. BAR SUPPORTS SHALL BE PLASTIC TIPPED OR STAINLESS STEEL

CONCRETE EXPOSED TO WEATHER SHALL BE AIR ENTRAINED TO 5% (+-1%) WITH AN ADMIXTURE THAT CONFORMS TO ASTM C-260.

SLAB ON GRADE NOTES

PROVIDE CONCRETE SLABS OVER POLYETHYLENE VAPOR BARRIER AND 4" OF POROUS FILL AS FOLLOWS: 4" SLAB REINFORCED WITH 6x6-W2.1xW2.1 WELDED WIRE FABRIC AND WITH 3,500 PSI MIX CONCRETE.

MAXIMUM SLUMP FOR CONCRETE SLABS WILL BE 5" WITH TYPE II CEMENT.

ALL WELDED WIRE FABRIC SHALL BE IN ACCORDANCE WITH ASTM A-185. LAP ADJOINING PIECES AT LEAST ONE FULL MESH. WELDED WIRE FABRIC SHALL BE ORDERED IN SHEETS, NOT ROLLS, WELDED WIRE FABRIC SHALL BE BLOCKED INTO POSITION WITH PRECAST CONCRETE BLOCKS HAVING THE SAME COMPRESSIVE STRENGTH OF THE SLAB.

THE ALTERNATE WIRES OF THE WELDED WIRE FABRIC MUST BE PRECUT AT THE SLAB CONTRACTION JOINT LOCATIONS TO CREATE A "WEAKENED PLANE".

THE USE OF POLYPROPYLENE FIBERS (IN LIEU OF WELDED WIRE FABRIC) IS PROHIBITED.

ALL POROUS FILL MATERIAL SHALL BE A CLEAN GRANULAR FILL MATERIAL WITH 100% PASSING THE 1 1/2 NO. 4 SIEVE. POROUS FILL SHALL BE COMPACTED TO 98% MAX DRY DENSITY PER ASTM D-1557 MODIFIED PROCTOR METHOD. " SIEVE AND NO MORE THAN 5% PASSING THE SLAB JOINTS SHALL BE FILLED WITH A SEALANT PER THE MANUFACTURER RECOMMENDATIONS.

SLABS EXPOSED TO WEATHER SHALL BE AIR ENTRAINED TO 5% (± 1%) WITH AN ADMIXTURE THAT CONFORMS TO ASTM C-260.

THE SLAB SHALL BE WET CURED BY KEEPING THE SLAB MOIST FOR A PERIOD OF SEVEN DAYS, ALTERNATIVELY, PROVIDE A WET-CURING SEALANT PER THE MANUFACTURERS RECOMMENDATIONS.

WRAP VAPOR BARRIER AROUND FOOTING ACCORDING TO BUILDING SCIENCE BEST PRACTICE.

FOUNDATION NOTES

ALL FOOTINGS SHALL BEAR ON UNDISTURBED, FIRM NATURAL SOIL, OR COMPACTED FILL CAPABLE OF SUPPORTING A DESIGN BEARING PRESSURE OF 3000 PSF. CONTRACTOR SHALL CONFIRM BEARING CAPACITY WITH INDEPENDENT TESTING.

REMOVE ANY SOFT OR FROZEN SOIL MATERIAL ENCOUNTERED UNDER FOOTINGS

UNLESS OTHERWISE NOTED, PROVIDE THE FOLLOWING COVER FOR FOUNDATION REINFORCEMENT BOTTOM BARS & BARS IN CONCRETE CAST AGAINST EARTH: 3" BARS THAT ARE EXPOSED TO WEATHER:

#5 OR SMALLER1 1/2" #6 OR BIGGER 2"

ALL BARS SHALL BE LAPPED 40 X THE BAR DIAMETER AT SPLICES.

PRIOR TO COMMENCING FOUNDATION WORK, COORDINATE WORK WITH UTILITIES.

A LAYER OF WELL-GRADED FREE-DRAINING GRANULAR MATERIAL/SAND (AT LEAST 6" THICK AND COMPACTED TO 98% OF SPMDD) SHOULD BE PLACED UNDER THE FOUNDATION TO PROVIDE A UNIFORM BEARING SURFACE.

TEMPORARY SHORING AND BRACING NOTES

THE CONTRACTOR IS RESPONSIBLE FOR THE STRENGTH, SAFETY, AND STABILITY OF THE NEW AND EXISTING STRUCTURE DURING CONSTRUCTION AND SHALL PROVIDE TEMPORARY SHORING, BRACING AND OTHER ELEMENTS REQUIRED TO MAINTAIN STABILITY UNTIL THE STRUCTURE IS COMPLETE. IT IS CONTACTOR'S RESPONSIBILITY TO BE FAMILIAR WITH THE WORK REQUIRED IN THE CONSTRUCTION DOCUMENTS AND THE REQUIREMENTS FOF EXECUTING IT PROPERLY. THE CONTRACTOR SHALL AT THIS DISCRETION EMPLOY AN SSE, A REGISTERED PROFESSIONAL ENGINEER FOR THE DESIGN OF ANY TEMPORARY BRACING AND SHORING

STRUCTURAL STEEL NOTES

ALL DESIGN, FABRICATION AND ERECTION SHALL CONFORM TO THE AISC STEEL CONSTRUCTION MANUAL, 13TH EDITION, WELDING SHALL CONFORM TO THE LATEST AWS AND AISC SPECIFICATIONS.

WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE BEST PRACTICE AND WITHIN THE TOLERANCES SPECIFIED IN THE AISC SPECIFICATIONS FOR STRUCTURAL STEEL

IT IS SPECIFICALLY NOTED THAT BURNED HOLES ARE NOT ACCEPTABLE UNLESS SPECIAL PERMISSION IS GIVEN BY ENGINEER.

ALL SHOP FABRICATED WORK SHALL BE DONE IN A SHOP APPROVED BY THE GOVERNING AGENCY. FABRICATOR SHALL SUBMIT PROGRAM OF WELDING INSPECTION TO ENGINEER FOR APPROVAL.

ALL STRUCTURAL STEEL SHALL BE AS FOLLOWS UNO: ALL WF, WT SHAPES A992 GRADE 50 CONNECTION PL & MISC STEEL (UNO) ASTM A36 GUSSET & COLLECTOR PLATES ASTM A572 GRADE 50 PIPE COLUMNS (TYPE S, SEAMLESS) ASTM A53 GRADE B STRUCTURAL TUBING ASTM A500 GRADE B ANGLE, CHANNELS ASTM A36 THREADED ROD ASTM A36 HEADED SHEAR STUDS ASTM A108 GRADES 1015 TO 1020,

TYPE 316, 50 ksi a) E70XX FOR A36 ELECTRODES

STEEL AND SMAW PROCESS OR EQUIVALENT. b) FOR OTHER STEEL G

RADES USE MATCHING WELD METAL AND

PROCESSES.

ALL HIGH STRENGTH BOLTS SHALL BE ASTM A325-N TYPE UNLESS OTHERWISE NOTED.

ALL BOLTS USED FOR ERECTION SHALL BE ASTM A325 TYPE WITH THREADS EXCLUDED FROM SHEAR PLANES.

ALL PLAIN ANCHORS SHALL BE A36; ALL ANCHOR BOLTS SHALL COMPLY WITH ASTM F1554. 3" MINIMUM CONCRETE COVER WILL BE PLACED AROUND ALL ANCHOR BOLTS EXPOSED TO THE WEATHER, U.N.O.

WELDING MATERIALS: PROVIDE TYPE REQUIRED FOR MATERIALS BEING WELDED, PER AWS D1.1.

1'-0" SCALE.

WELD LENGTHS CALLED FOR ON PLANS ARE THE NET EFFECTIVE LENGTH REQUIRED. WHERE FILLET WELD SYMBOL IS GIVEN WITHOUT INDICATION OF SIZE, USE MINIMUM SIZE WELDS AS SPECIFIED IN AISC MANUAL OF STEEL CONSTRUCTION LATEST EDITION. THIS INCLUDES OPEN WEB JOIST CONNECTIONS.

WRITTEN WELDING PROCEDURE SPECIFICATIONS (WPS) PER THE RECOMMENDATIONS OF THE AMERICAN WELDING SOCIETY (AWS) SHALL BE DEVELOPED BY THE FABRICATOR/ERECTOR AND SUBMITTED FOR REVIEW BY THE ENGINEER PRIOR TO ANY WELDING OF THE STRUCTURAL STEEL. THE WELDING PROCEDURES SHALL INCLUDE ALL THE WELDED JOINTS AND CONFIGURATIONS TO BE USED ON THIS PROJECT-ONLY WPS WHICH ARE RELEVANT TO THIS PROJECT SHALL BE SUBMITTED. ALL WELDED JOINTS SHALL BE PRE-QUALIFIED PER AWS OR BE QUALIFIED BY TEST PER AWS. A PROCEDURE QUALIFICATION RECORD (PQR) SHALL BE INCLUDED WITH THE WPS IF THE WELDING PROCEDURE OR JOINT IS QUALIFIED BY TESTING. THE ELECTRODE MANUFACTURER AND PRODUCT/TRADE NAME SHALL BE IDENTIFIED IN THE WPS IN ADDITION TO THE AWS ELECTRODE CLASSIFICATION NAME. A COPY OF THE ELECTRODE MANUFACTURER'S TECHNICAL DATA SHEETS WITH THE RECOMMENDED WELDING PARAMETERS SHALL BE SUBMITTED WITH THE WPS.

DO NOT WELD ANY STRUCTURAL STEEL MEMBER OR CONNECTION UNLESS EXPLICITLY CALLED OUT IN THE CONTRACT DOCUMENTS.

WELD SYMBOLS SHOW FINAL WELD REQUIRED. THE CHOICE TO WELD IN THE FIELD OR IN THE SHOP SHALL BE UP TO THE CONTRACTOR AND SHALL BE INDICATED IN THE FABRICATOR'S SHOP DRAWINGS.

THIS STRUCTURE IS CONSIDERED A NON-SELF-SUPPORTING STEEL FRAME. THE CONTRACTOR SHALL PROVIDE ADEQUATE TEMPORARY SUPPORTS UNTIL ALL PERMANENT SHEAR WALLS, MOMENT FRAMES, BRACED FRAMES, AND FLOOR SLABS ARE IN PLACE.

Fb=900 PSI, Fv =180 E=1,600,000 PSI,

PLAN.

PROVIDE CONTINUOUS INSPECTION FOR ALL FABRICATION AND WELDING OF STRUCTURAL STEEL AS REQUIRED PER CODE REQUIREMENTS. ALL COMPLETE PENETRATION GROOVE WELDS IN JOINTS AND SPLICES SHALL BE TESTED 100 PERCENT IN ACCORDANCE WITH IBC . USE ONE OF THE APPROVED METHODS OF TIGHTENING HIGH STRENGTH BOLTS.

A WELDING SEQUENCE SHALL BE PLANNED TO MINIMIZE **RESIDUAL STRESSES AND DISTORTIONS OF INDIVIDUAL** MEMBERS AND THE BUILDING FRAME. ALL DETAILING, FABRICATION, AND ERECTION SHALL COMPLY WITH AISC, LATEST EDITION.

UNLESS OTHERWISE NOTED. ALL STIFFENER PLATES ARE 3/8" THICK MINIMUM AND ALL BUTT WELDS ARE FULL PENETRATION WELDS. ERECTION CLIPS, TEMPORARY BRACING, ETC., REQUIRED BY THE CONTRACTOR ARE NOT SHOWN.

SUBMIT SHOP DRAWINGS FOR THE FABRICATION AND ERECTION OF ALL ASSEMBLIES OF STRUCTURAL STEEL WORK. INCLUDE PLANS AND ELEVATIONS AT NOT LESS THAN 1/4" TO 1'-0" SCALE, AND INCLUDE DETAILS OF SECTIONS AT NOT LESS THAN 1" TO

NO FINISH FABRICATION SHALL BE COMMENCED OR MATERIAL DELIVERED TO THE JOB UNTIL THE ENGINEER HAS REVIEWED AND APPROVED THE SHOP DRAWINGS.

ALL STRUCTURAL STEEL SHALL BE PAINTED WITH ONE SHOP COAT OF ZINC CHROMATE PRIMER OR EQUAL. AFTER ERECTION, FIELD CONNECTIONS SHALL BE TOUCHED UP. DO NOT PAINT PORTION OF STEEL TO BE EMBEDDED IN CONCRETE, HEADED ANCHOR STUDS, FAYING SURFACES OR AREAS TO RECEIVE FIRE PROOFING, EXTERIOR, EXPOSED STEEL MEMBERS ARE SPECIFIED TO BE HOT-DIPPED GALVANIZED OR STAINLESS AS NOTED.

THE USE OF E70T-4 WELDING WIRE IS NOT ALLOWED FOR ANY APPLICATION.

ALL STRUCTURAL STEEL SHALL BE PROPERLY GUYED AND BRACED UNTIL FLOOR AND ROOF FRAMING SYSTEM AND LATERAL LOAD RESISTING SYSTEM IS IN PLACE.

ALL BEAM CONNECTIONS SHALL BE DETAILED TO PROVIDE A SHEAR CONNECTION WITH A MINIMUM DESIGN CAPACITY AS THAT SHOWN IN THE DRAWINGS BEAM SHEAR TAB CONNECTION TABLE FOR THE CORRESPONDING BEAM SIZE.

ALL CONNECTIONS NOT DETAILED OR OTHERWISE NOTED SHALL BE DESIGNED AS AISC TYPE 2 BOLTED CONNECTIONS DESIGNED FOR FULL LOAD CAPACITY OF THE CONNECTING MEMBERS.

ALL BOLTS IN MOMENT CONNECTIONS SHALL BE BEARING TYPE CONNECTIONS.

ALL WELDS SHALL BE MADE BY WELDERS CERTIFIED ACCORDING TO AWS PROCEDURES.

WOOD NOTES

1. LUMBER SHALL CONFORM TO THE LATEST EDITION OF THE "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION" AS PUBLISHED BY THE NATIONAL FOREST PRODUCTS ASSOCIATION. UNLESS OTHERWISE NOTED.

2. ALL LUMBER TO BE DOUGLAS FIR # 2 OR BETTER, CAPABLE OF DEVELOPING THE FOLLOWING MINIMUM ALLOWABLE STRESSES:

3. LVL SHALL BE PARALLAM LVL OR SIMILAR

4. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS ON THE

LATERAL FRAMING NOTES:

PLAN.

1. THE STRUCTURAL DESIGN OF THIS RESIDENCE WAS PERFORMED IN COMPLIANCE WITH THE INTERNATION BUILDING CODE.

2. FRAMING COMPONENTS AND FASTENERS AS IDENTIFIED IN THESE DRAWINGS AND NOTES ADEQUATELY RESIST THE LATERAL LOAD REQUIREMENTS AS DEFINED BY THE INTERNATIONAL BUILDING CODE.

3. ALL EXTERIOR WALLS TO FOLLOW SHEARWALL SHEATING CRITERIA.

4. SHERWALLS CONSTRUCTION: SHEATHING TO BE 1/2 " APA RATED SHEATHING TO BE ATTACHED TO THE WALL STUDS WITH 8dNAILS @ 4" OC AROUND EDGES & 8" OC IN FIELDS.

5. ALL PLYWOOD SEAMS IN A SHEARWALL SHALL BE **BLOCKED WITH DIMENSIONAL** LUMBER OF THE SAME SIZE AS THE WALL STUDS.

6. REFER TO PLANS AND SECTIONS FOR STUD SIZES, STUDS SHALL BE SPACED AT 16 INCHES ON CENTER UNLESS NOTED OTHERWISE ON

7. CARE SHOULD BE TAKEN TO ADJUST NAIL GUN PRESSURE SO AS TO NOT OVER DRIVE NAILS INTO PLYWOOD. NAIL HEADS SHOULD BE FLUSH WITH PLYWOOD FACE. OVER DRIVING NAILS GREATLY REDUCES THE EFFECTIVENESS OF THE 'SHEARWALL.

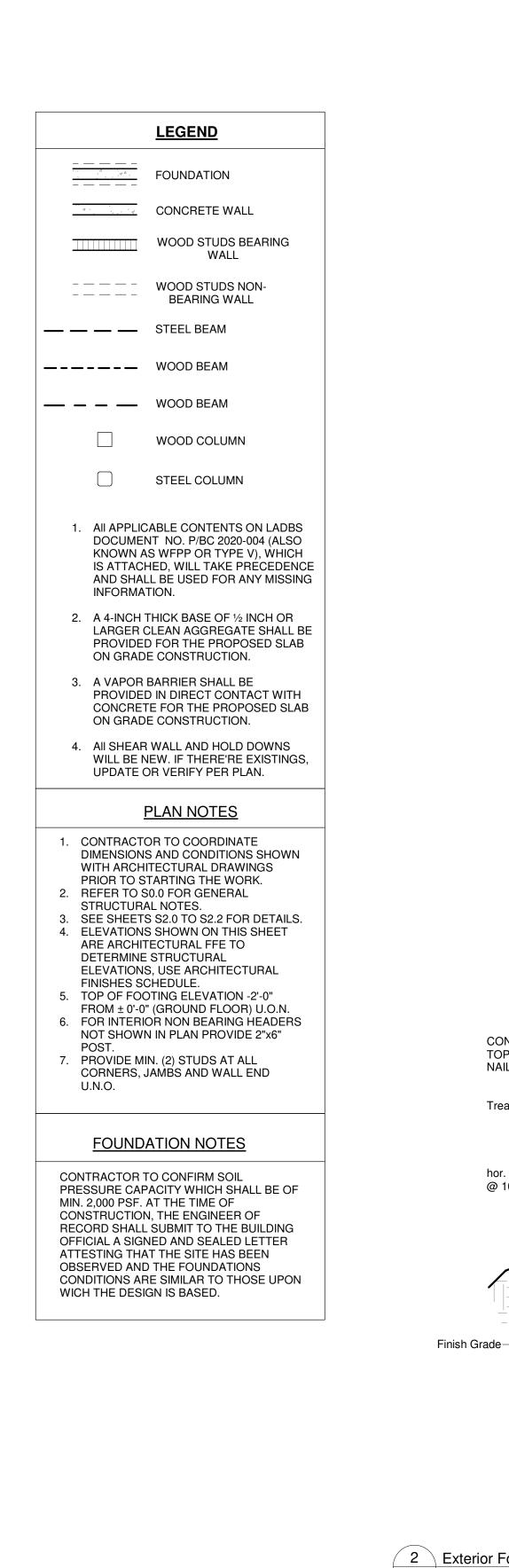
8. FOR FRAMING SIZES REFER TO FRAMING PLANS.

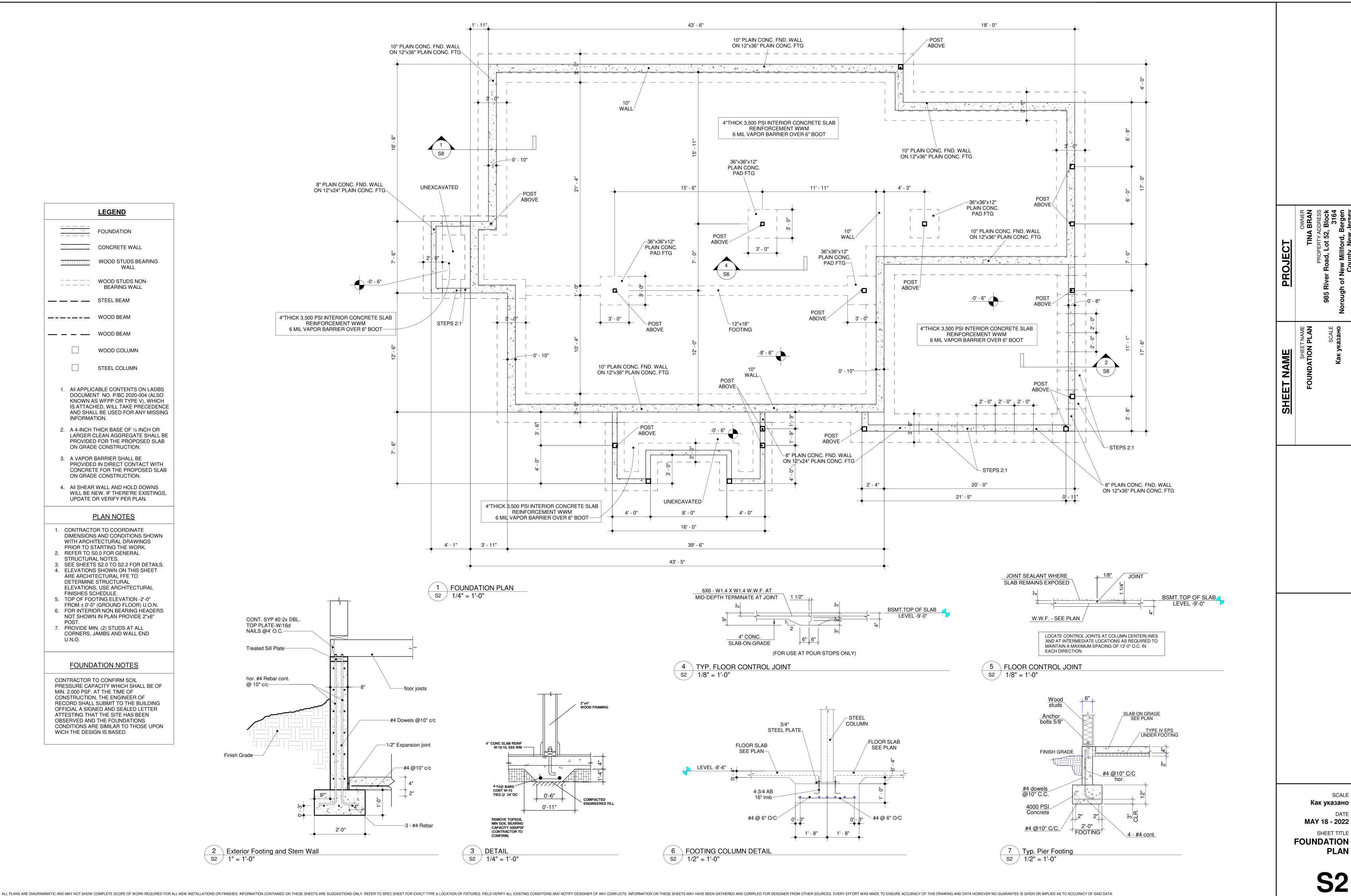
HANGER SELECTION TABLE							
QT MEMBER	1	2	3				
2X8	LUS28	LUS28-2	LUS28-3				
2X10	LUS210	LUS210-2	LUS210-3				
2X12	LUS210	LUS210-2	LUS210-3				
9 1/2" LVT	HU9	HU410	HU610				
11 7/8" LVT	HU11	HU412	HU612				
14 LVT	HU14	HU416	HU616				
2 5/16" FLG I-JOIST	IUS 2.37						
2 1/2" FLG I-JOIST	IUS 2.56						
3 1/2" FLG I-JOIST	IUS 3.56						

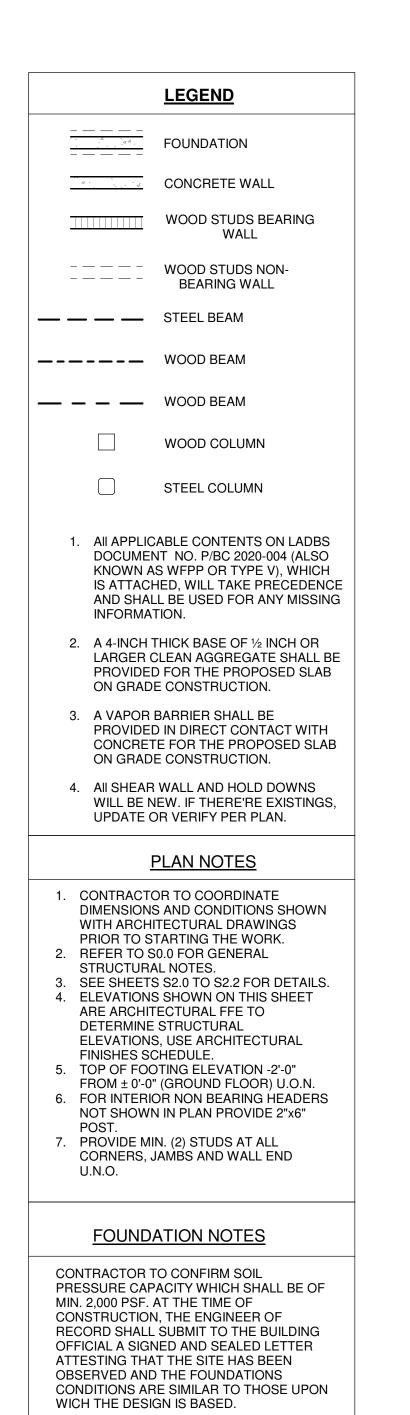
NOTE 1. USE HANGERS ABOVE FOR PROPOSED STRUCTURE UNLESS OTHERWISE NOTED ON FRAMING PLANS. 2. INSTALL ALL HANGERS WITH MAXIMUM NUMBER OF FASTENERS.

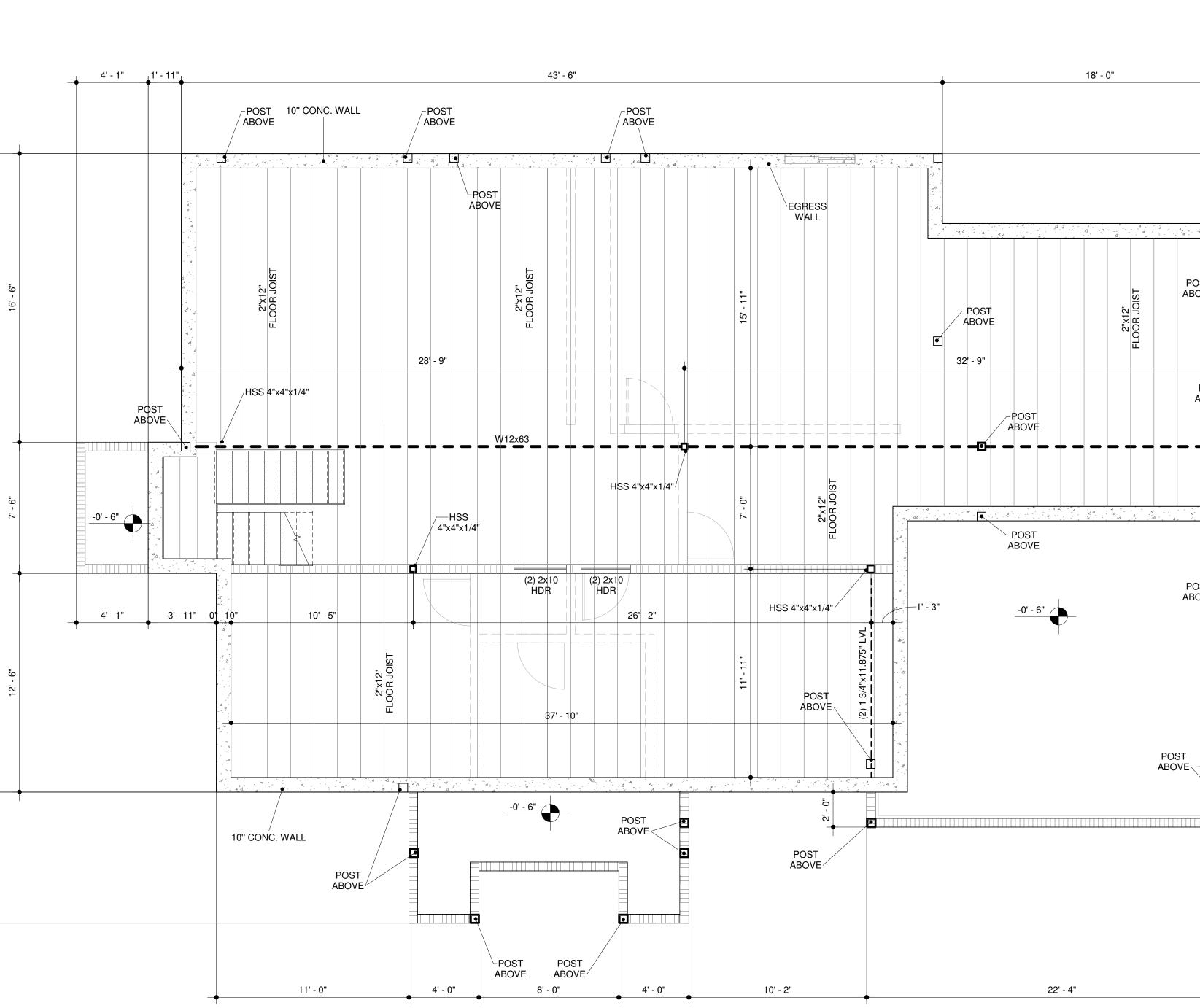
> * ALL DIMENSIONS TO BE VERIFIED IN FIELD. * ALL EXIST DIMENSIONS SUBMITTED BY THE **CLIENT** * FASTENING REQUIREMENTS FOR WOOD TOGETHER TO BE ACCORDING THE MANUFACTURE RECOMMENDATIONS.

PROJECT	OWNER TINA BRAN	PROPERTY ADDRESS 985 River Road, Lot 52, Block 3164	Norough of New Millford, Bergen County, New Jersey
SHEET NAME	SHEET NAME GENERAL NOTIES	SCALE	
		Y 18 - 2	TITLE
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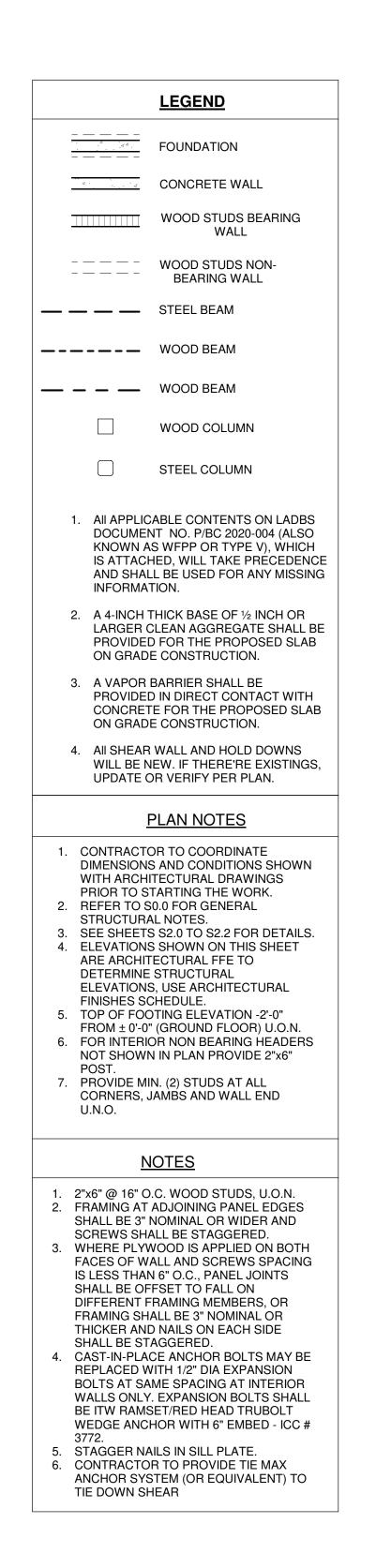


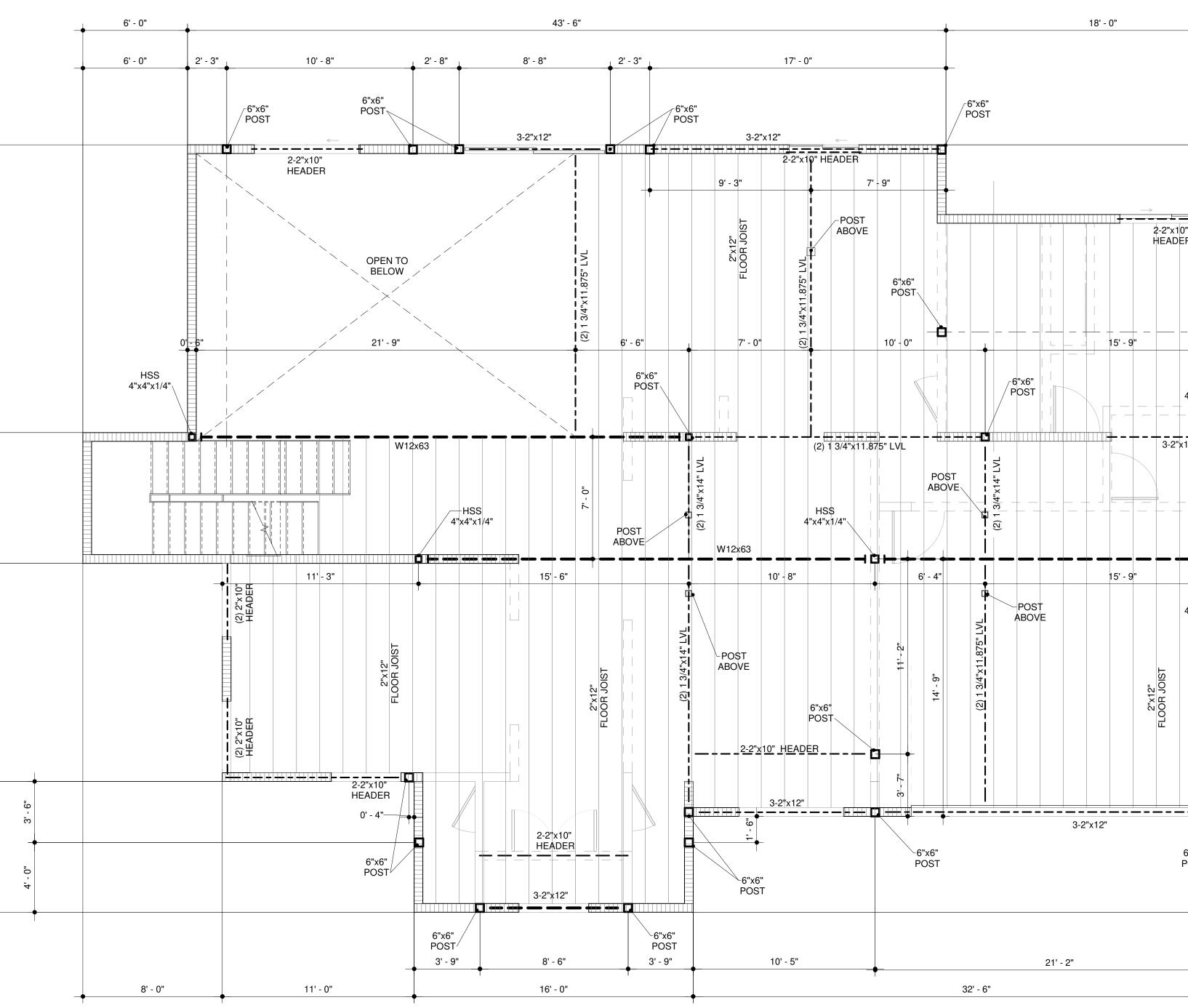




BASEMENT PLAN 1/4" = 1'-0"

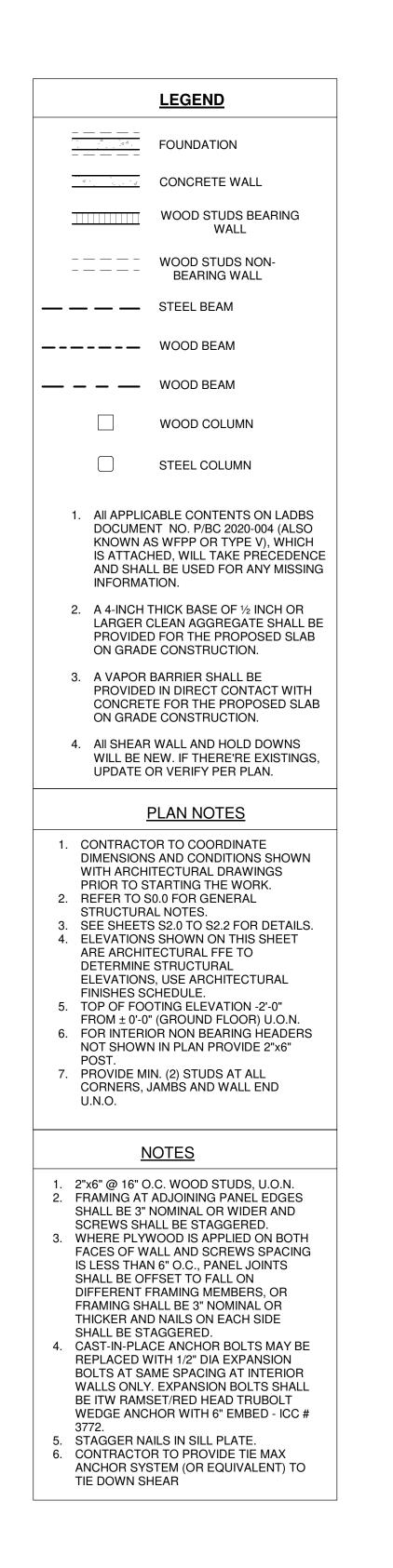
POST ABOVE	PROJECT	OWNER TINA BRAN	PROPERTY ADDRESS 985 River Road, Lot 52, Block 3164 Norouch of New Millford Berger	County. New Jersey
	SHEET NAME	SHEET NAME BASEMENT PLAN	SCALE 1/4" = 1'-0"	
		MA	SCA 1/4" = 1'- DA Y 18 - 20 SHEET TIT SEMEN PLA	TE 22 LE IT

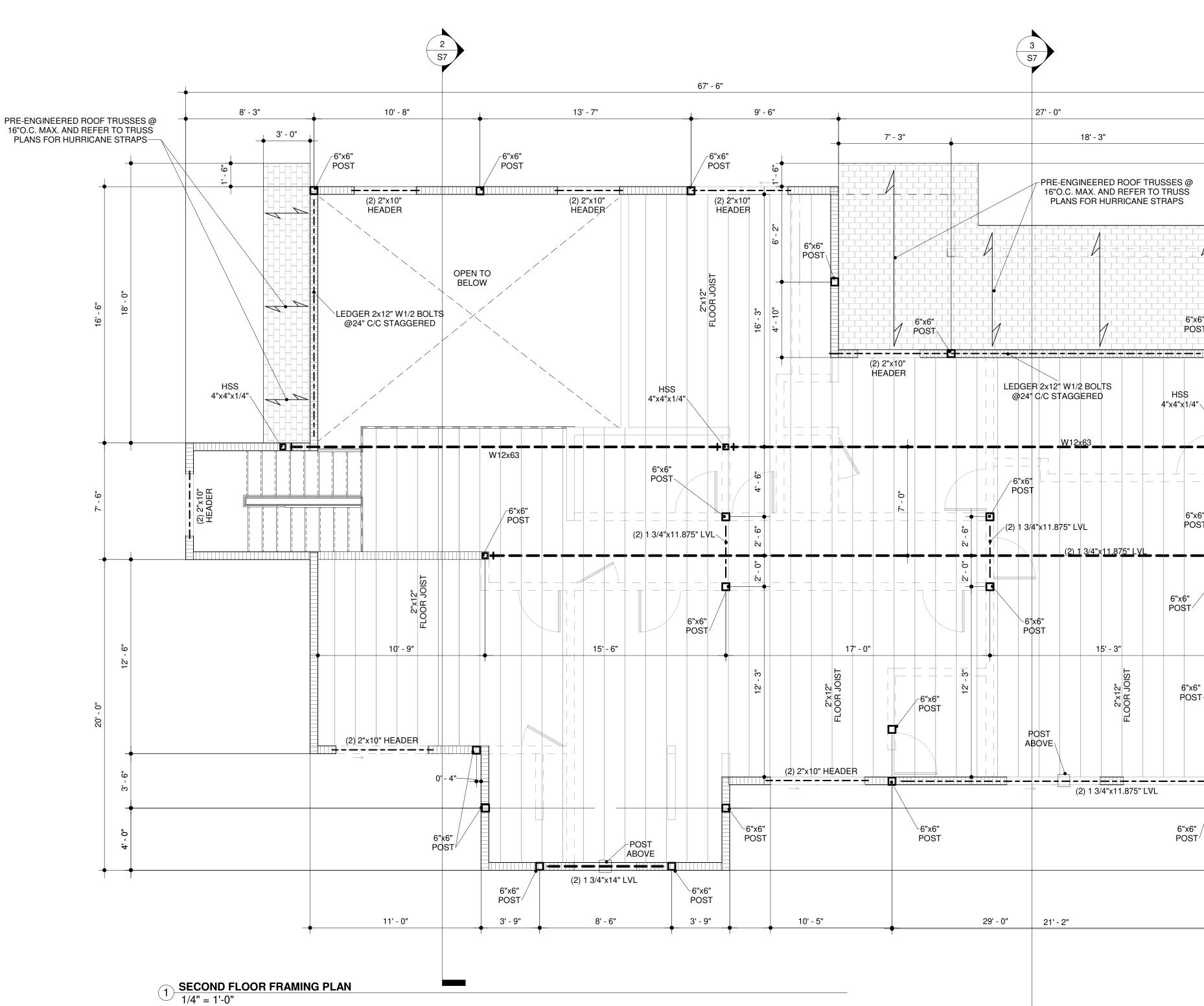




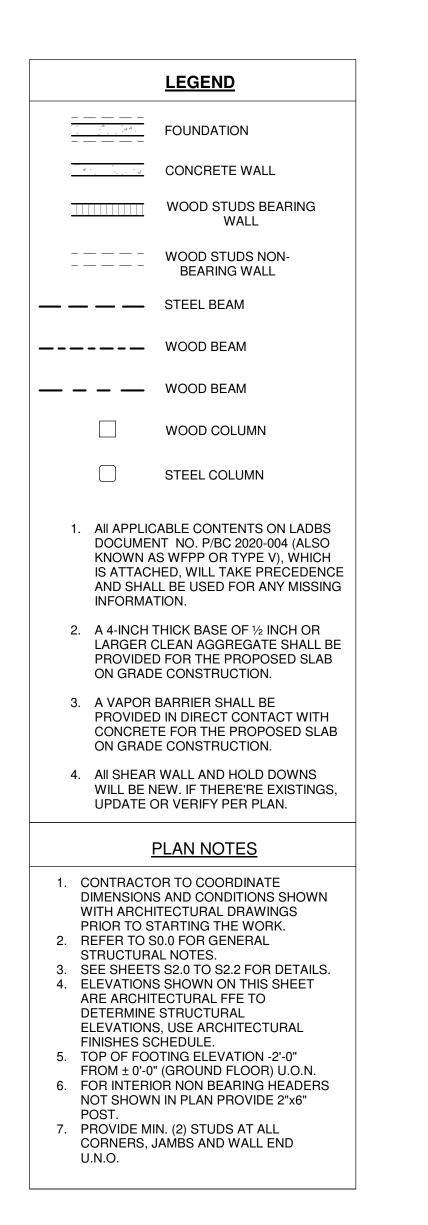
FIRST FLOOR FRAMING PLAN1/4" = 1'-0"

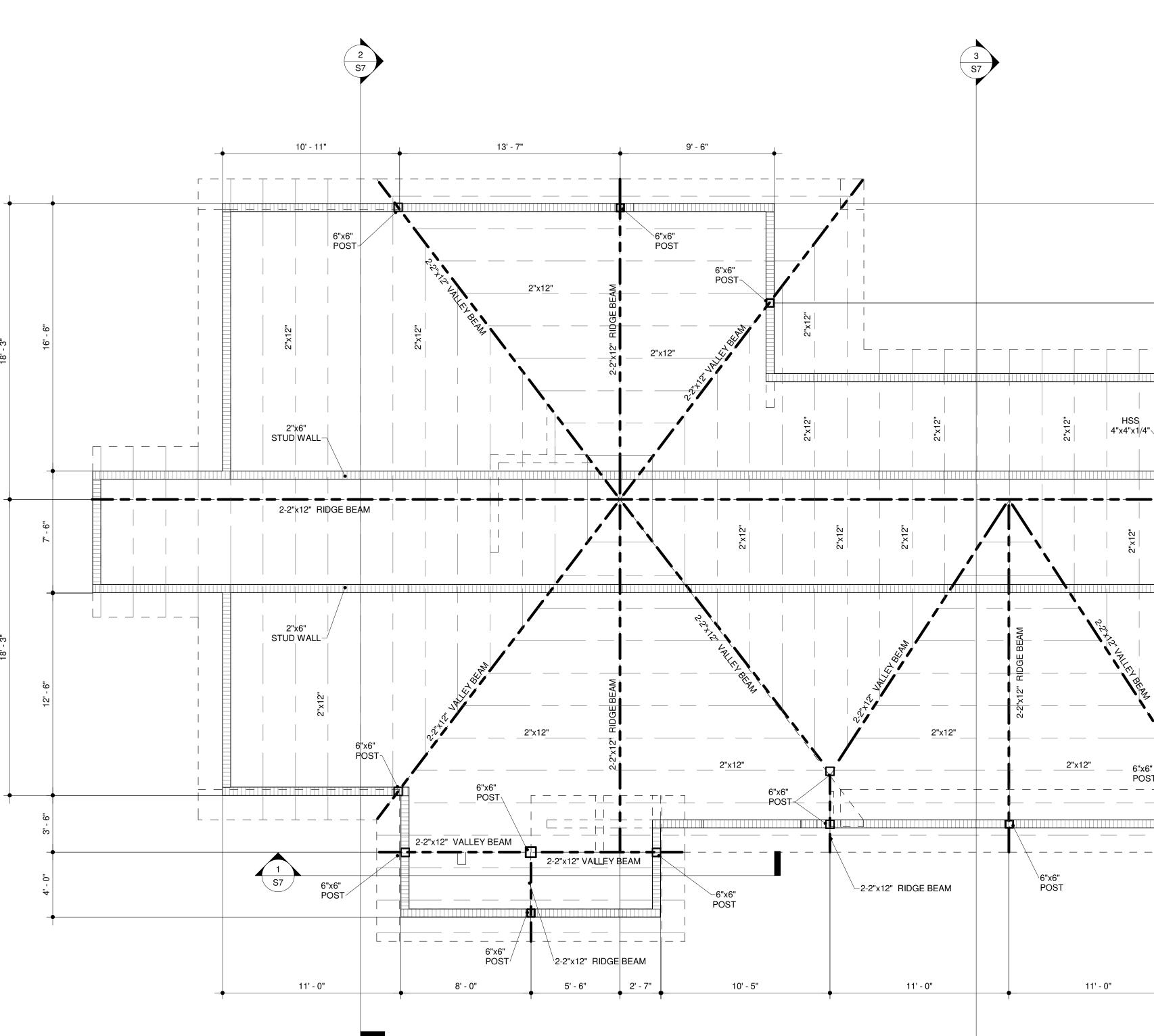
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	7' - 0"	44' - 0"
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:6" ST		6"x6" POST





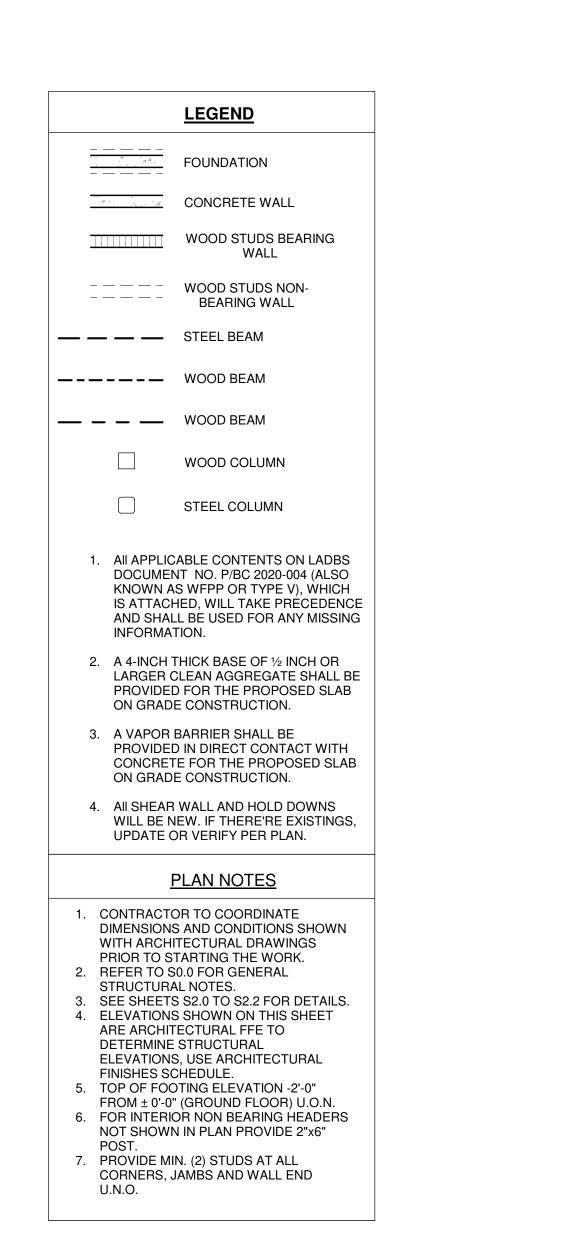
PROJECT	OWNER TINA BRAN	PROPERTY ADDRESS 985 River Road, Lot 52, Block 3164 Norough of New Millford, Bergen County. New Jersey
SHEET NAME	SHEET NAME SECOND FLOOR FRAMING PLAN	SCALE 1/4" = 1'-0"
SE	MA CONE	SCALE 1/4" = 1'-0" DATE Y 18 - 2022 SHEET TITLE D FLOOR NG PLAN

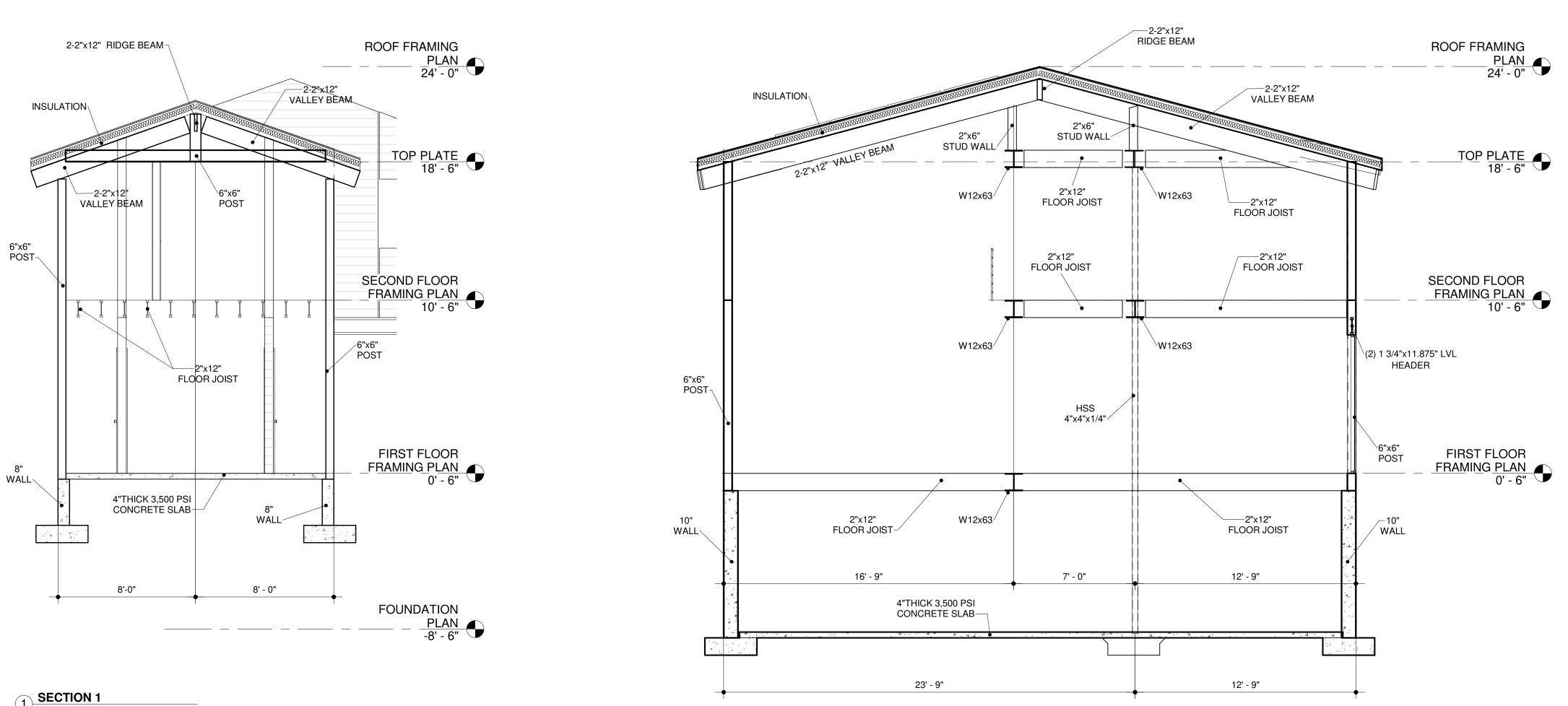




1 **ROOF FRAMING PLAN** 1/4" = 1'-0"

- 5" - 2"		PROJECT	OWNER TINA BRAN	PROPERTY ADDRESS 985 River Road, Lot 52, Block 3164
7' - 9" 4' - 4"		SHEET NAME	SHEET NAME ROOF FRAMING PLAN	
16 [°] - 6"				
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				sc 1/4'' = 1
		R	MA	D SHEET T RAMII PL





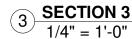
1 **SECTION 1** 1/4" = 1'-0"

2"x4" STUD WALL

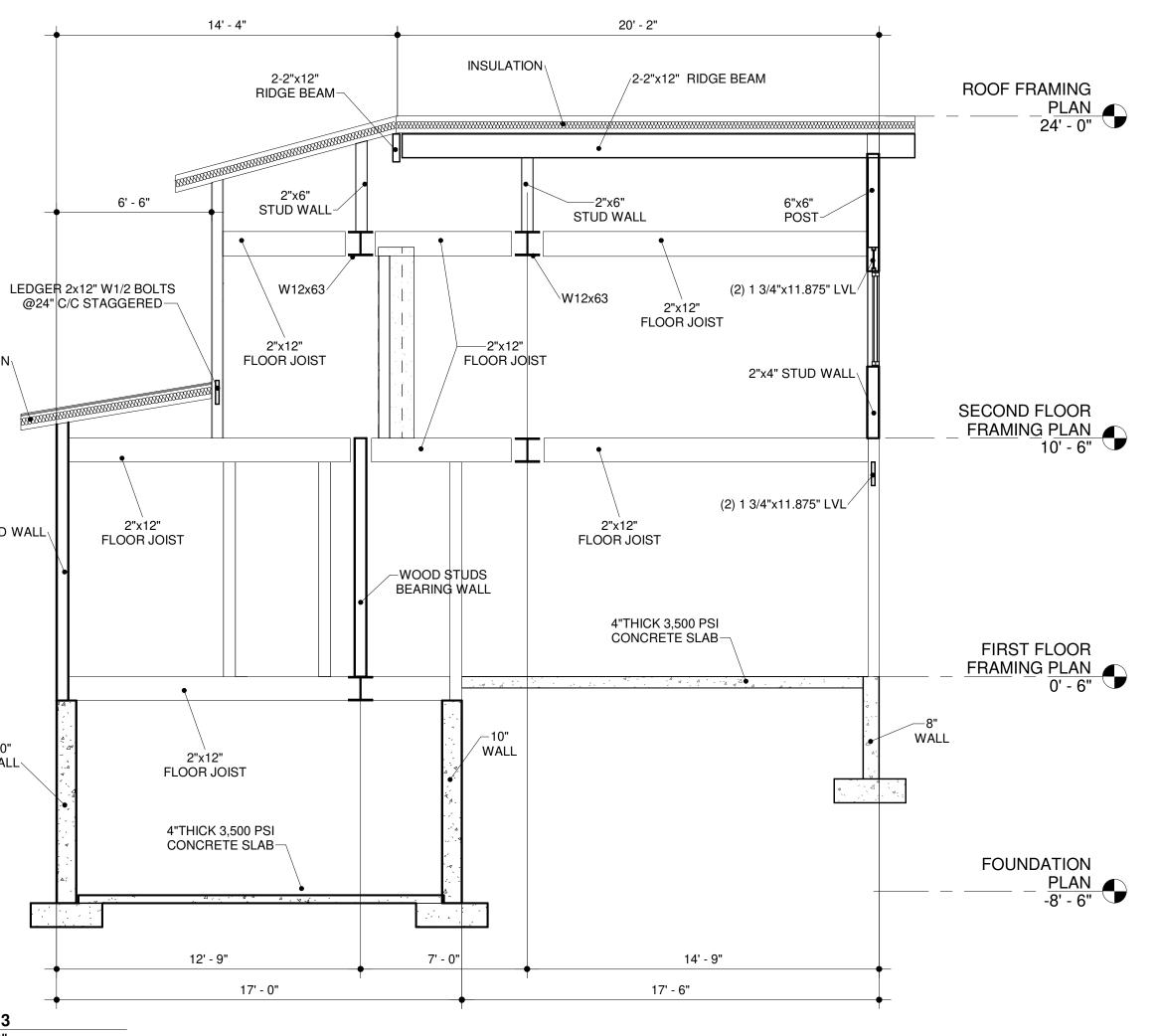
10"

WALL

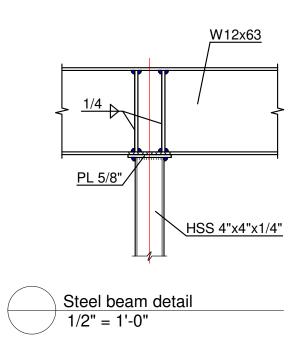
INSULATION

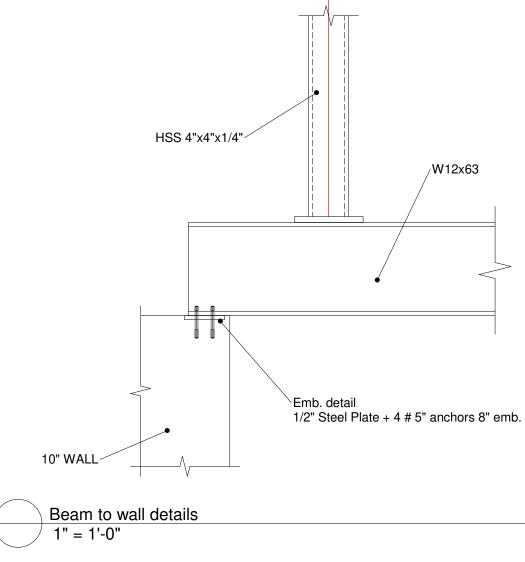


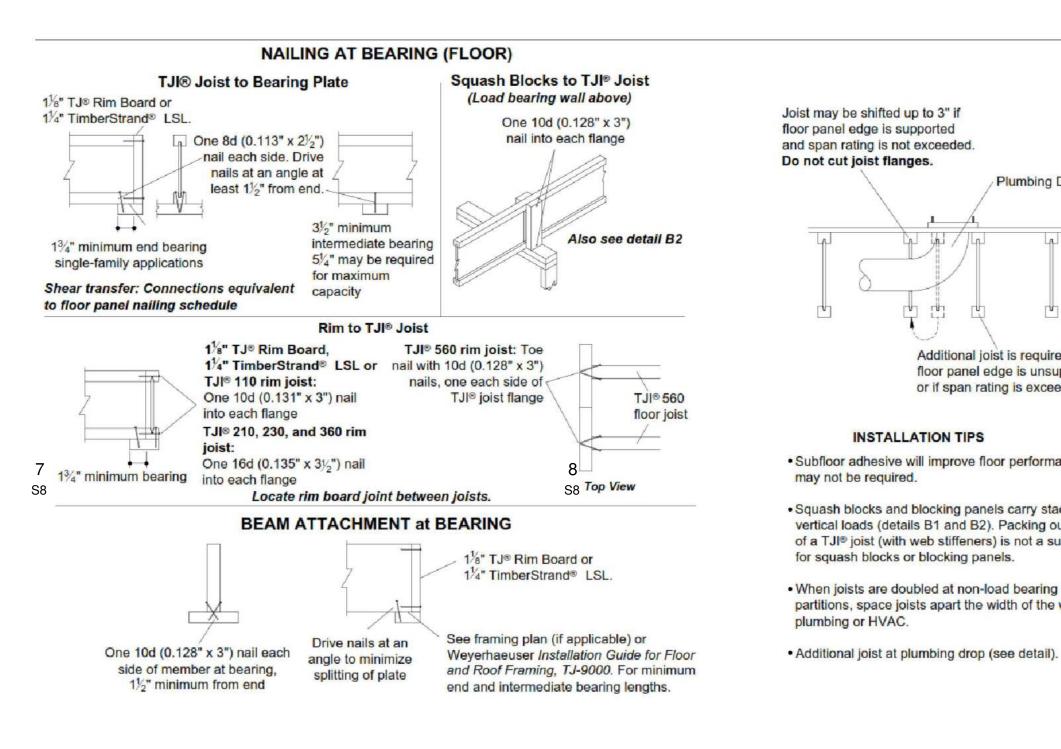
2 SECTION 2 1/4" = 1'-0"



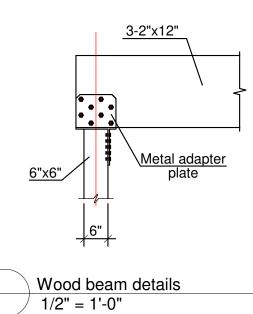
PROJECT	OWNER TINA BRAN	985 River Road, Lot 52, Block	Norough of New Millford, Bergen County, New Jersey
SHEET NAME	SHEET NAME SECTION	SCALE	1/4" = 1'-0"
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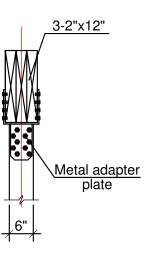




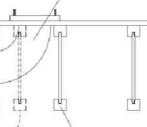


Load bearing or braced/shear wall above (must stack over wall below when present) Blocking panel: $1\frac{1}{8}$ " TJ[®] Rim Board, 1¹/₄" TimberStrand[®] LSL or TJI® joist Web stiffeners required on both sides of both joist ends End of joists at centerline of support





Plumbing Drop



Additional joist is required if floor panel edge is unsupported or if span rating is exceeded.

· Subfloor adhesive will improve floor performance, but

 Squash blocks and blocking panels carry stacked vertical loads (details B1 and B2). Packing out the web of a TJI® joist (with web stiffeners) is not a substitute

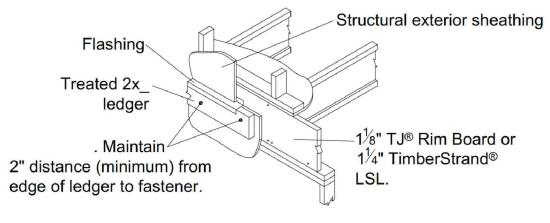
 When joists are doubled at non-load bearing parallel partitions, space joists apart the width of the wall for

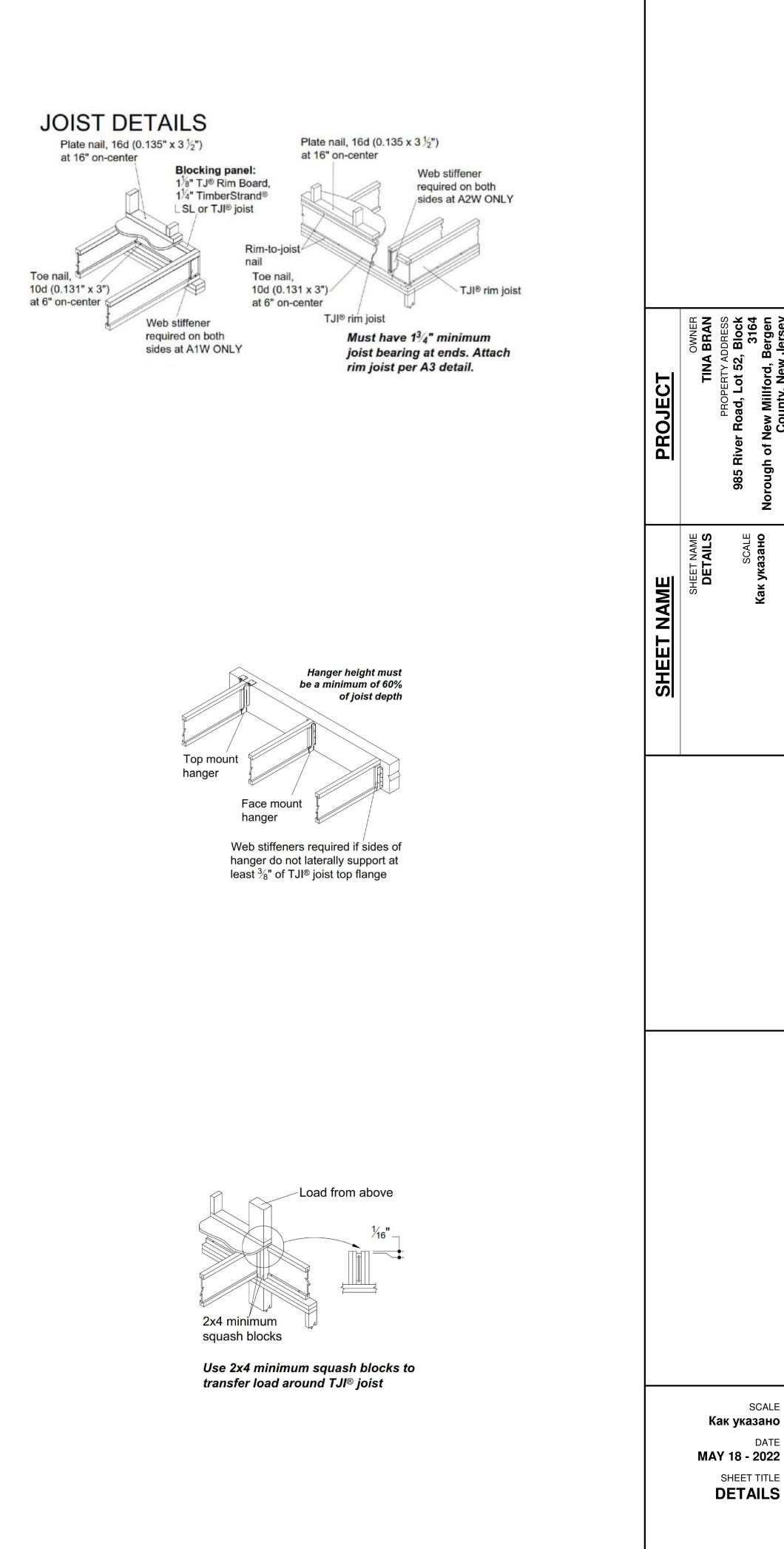
Double TJI[®] Joist Filler Block: Attach per the table. Clinch nails when possible.

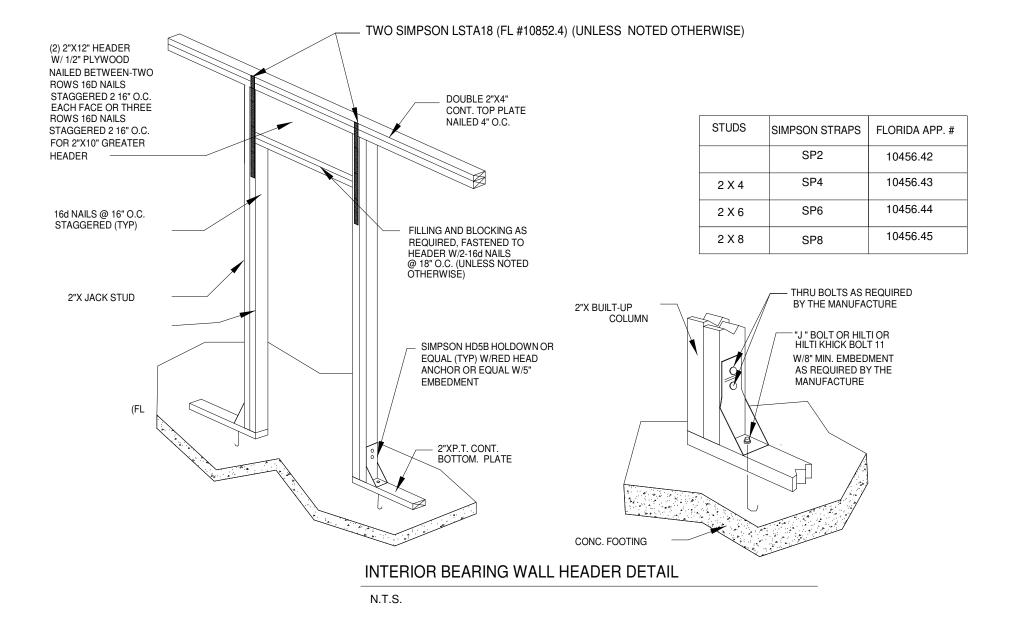
Hanger Backer Block: Install tight to top flange (tight to bottom flange with face mount hangers). Clinch nails when possible.

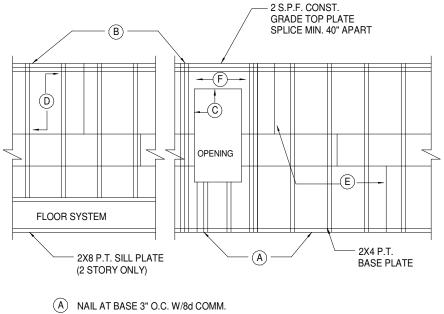
> Backer block both sides of web with single TJI® joist

EXTERIOR DECK ATTACHMENT

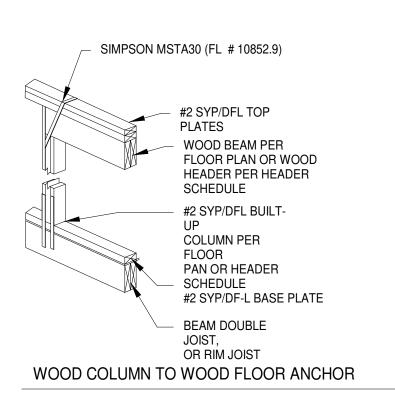








- (B) NAIL AT TOP PLATE 3" O.C. W/8d COMM.
- (C) NAIL OPENING PERIMETER @ 3" O.C. W/8D NAILS
- D NAIL AT 6" O.C. W/8d NAILS
- (E) STAGGER ALL VERT. JOINTS & NAIL @ 4" O.C. W/8d
- (F) NO PLYWOOD SPLICE @ HEADER-NAIL PLYWOOD TO HEADER W/8d @ 4" O.C. OPENINGS OVER 6'-0" USE SIMPSON LSTA 30 (FL# 10852.4) W/22-10d @ EA. END OF HEADER TO JACK @ STUD



SIMPSON LSTA 18

2"x4" STUDS TYP.

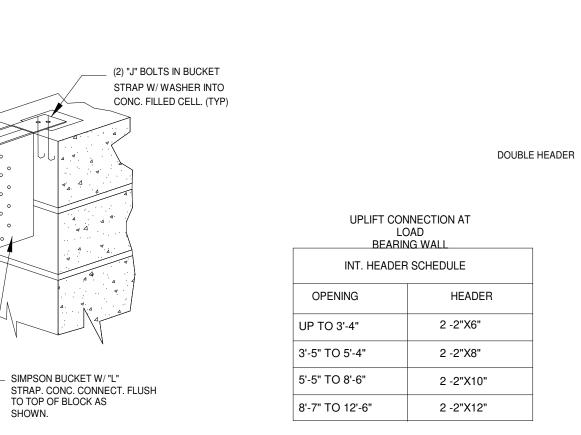
SIMPSON HD5B

P.T. BOTTOM PLATE

ANCHOR BOLT (5/8)

16 @ NAILS

@ 16" O.C. EACH FACE



SUPPORT BEAM/HEADER DETAIL

N.T.S. (USE TRIPLE 2X12 FOR SPANS LONGER THAN 12' ON CENTER)

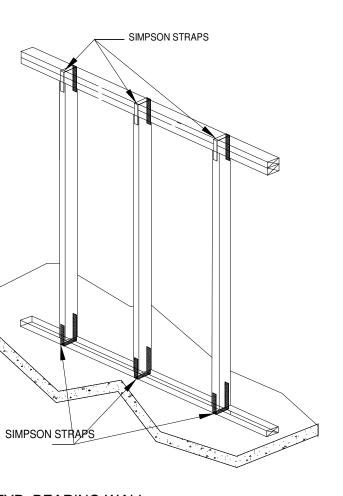
EXTERIOR WALL HEADER DETAIL

N.T.S.

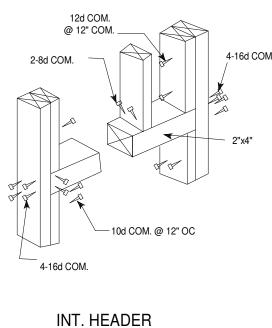
DBL. PT. 2X12'S NAILED

W/GALV. NAILS @16"

O.C. STAGGERED.



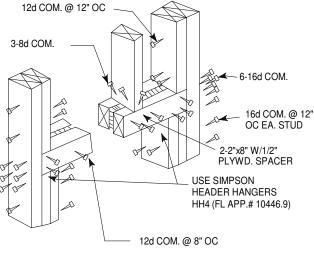


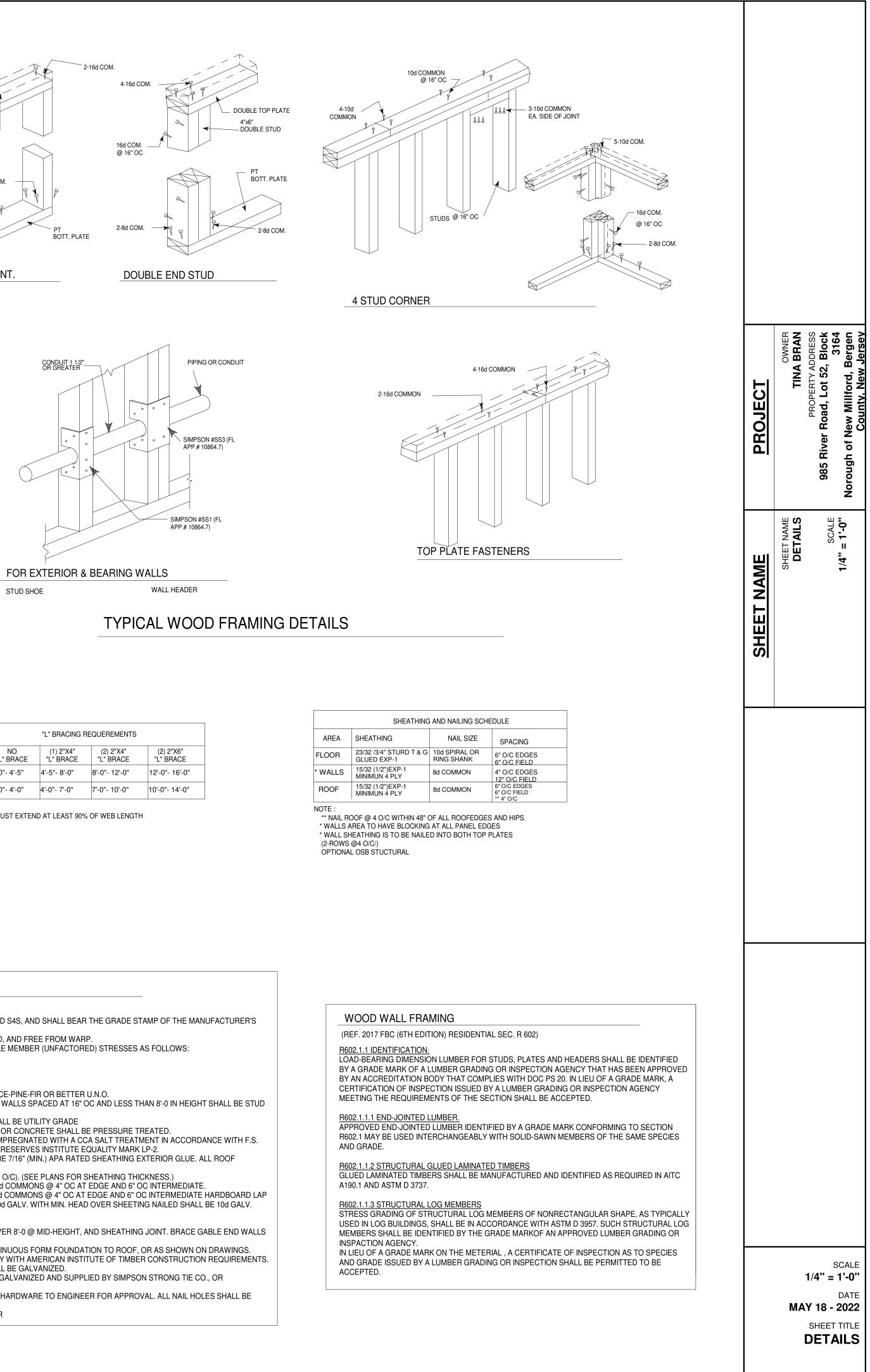


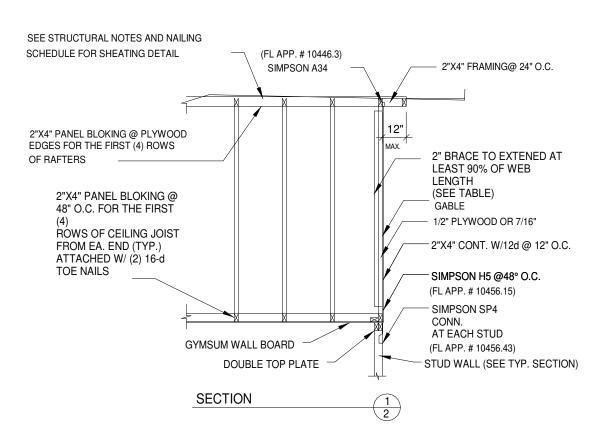
(NON-BEARING)

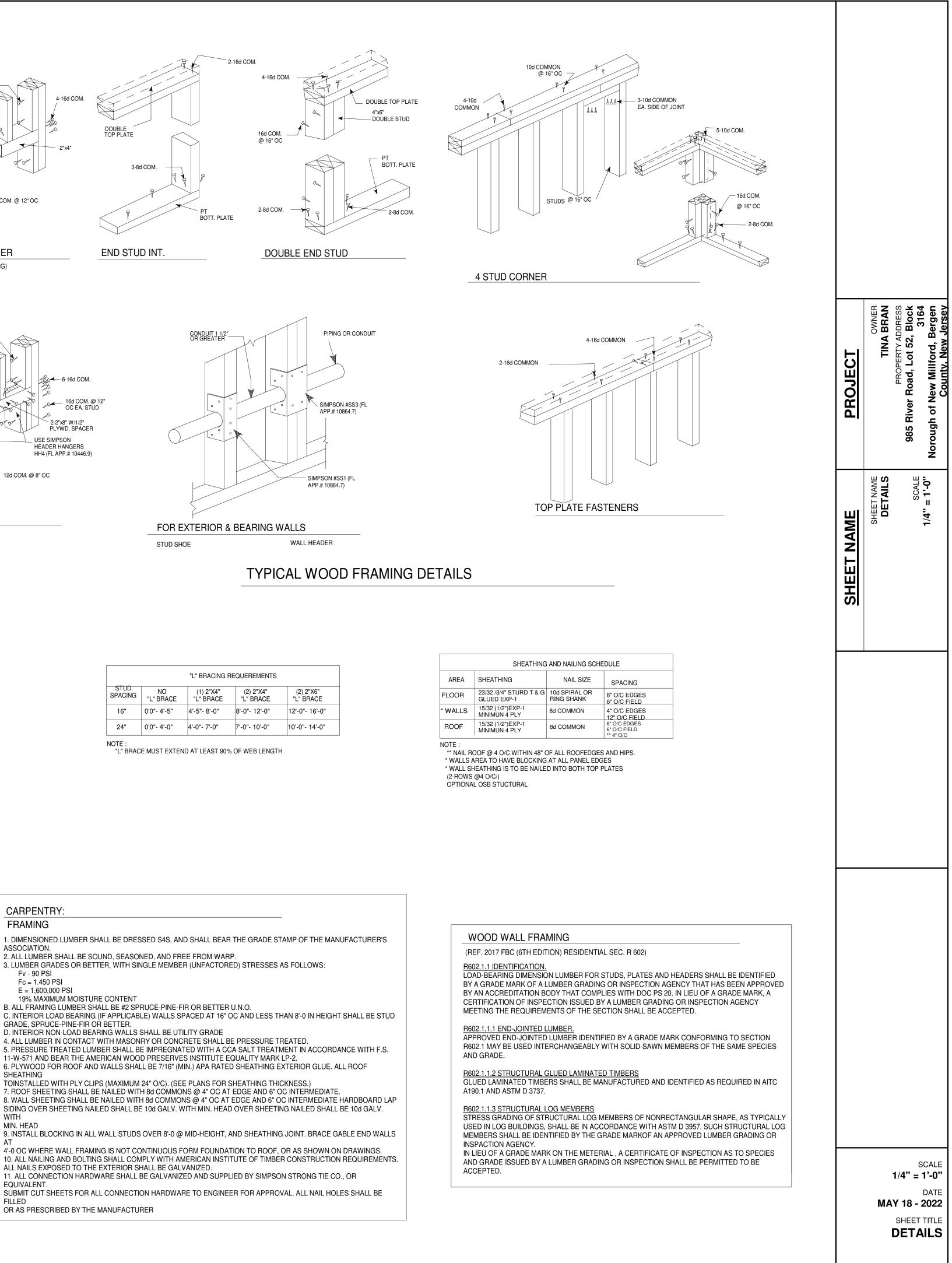
PLYWD. SPACER

EXT./BEARING



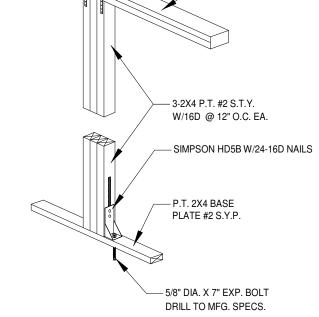






- 4- SIMPSON H6(FL # 10456.16)W/16-8d EA. – 2-2X4 S.P.F. TOP PLATE

GIRDER/BEAM



FRAME WALL GIRDER/BEAM DETAIL

CARPENTRY:

FRAMING

1. DIMENSIONED LUMBER SHALL BE DRESSED S4S, AND SHALL BEAR THE GRADE STAMP OF THE MANUFACTURER'S ASSOCIATION.

2. ALL LUMBER SHALL BE SOUND, SEASONED, AND FREE FROM WARP.

- Fv 90 PSI
- Fc = 1.450 PSI E = 1.600.000 PSI
- **19% MAXIMUM MOISTURE CONTENT**
- B. ALL FRAMING LUMBER SHALL BE #2 SPRUCE-PINE-FIR OR BETTER U.N.O. C. INTERIOR LOAD BEARING (IF APPLICABLE) WALLS SPACED AT 16" OC AND LESS THAN 8'-0 IN HEIGHT SHALL BE STUD GRADE, SPRUCE-PINE-FIR OR BETTER.
- D. INTERIOR NON-LOAD BEARING WALLS SHALL BE UTILITY GRADE
- 4. ALL LUMBER IN CONTACT WITH MASONRY OR CONCRETE SHALL BE PRESSURE TREATED.
- 11-W-571 AND BEAR THE AMERICAN WOOD PRESERVES INSTITUTE EQUALITY MARK LP-2. 6. PLYWOOD FOR ROOF AND WALLS SHALL BE 7/16" (MIN.) APA RATED SHEATHING EXTERIOR GLUE. ALL ROOF SHEATHING

TOINSTALLED WITH PLY CLIPS (MAXIMUM 24" O/C). (SEE PLANS FOR SHEATHING THICKNESS.) 7. ROOF SHEETING SHALL BE NAILED WITH 8d COMMONS @ 4" OC AT EDGE AND 6" OC INTERMEDIATE. 8. WALL SHEETING SHALL BE NAILED WITH 8d COMMONS @ 4" OC AT EDGE AND 6" OC INTERMEDIATE HARDBOARD LAP SIDING OVER SHEETING NAILED SHALL BE 10d GALV. WITH MIN. HEAD OVER SHEETING NAILED SHALL BE 10d GALV. WITH

MIN. HEAD 9. INSTALL BLOCKING IN ALL WALL STUDS OVER 8'-0 @ MID-HEIGHT, AND SHEATHING JOINT. BRACE GABLE END WALLS

AT 4'-0 OC WHERE WALL FRAMING IS NOT CONTINUOUS FORM FOUNDATION TO ROOF, OR AS SHOWN ON DRAWINGS. 10. ALL NAILING AND BOLTING SHALL COMPLY WITH AMERICAN INSTITUTE OF TIMBER CONSTRUCTION REQUIREMENTS. ALL NAILS EXPOSED TO THE EXTERIOR SHALL BE GALVANIZED.

11. ALL CONNECTION HARDWARE SHALL BE GALVANIZED AND SUPPLIED BY SIMPSON STRONG TIE CO., OR EQUIVALENT.

SUBMIT CUT SHEETS FOR ALL CONNECTION HARDWARE TO ENGINEER FOR APPROVAL. ALL NAIL HOLES SHALL BE FILLED OR AS PRESCRIBED BY THE MANUFACTURER

N.T.S.