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STRUCTURAL NOTES

GENERAL

THE STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH THE DRAWINGS OF ALL OTHER DISCIPLINES AND THE SPECIFICATIONS. THE CONTRACTOR SHALL VERIFY THE REQUIREMENTS OF OTHER TRADES AS TO INSERTS, ANCHORS, SLEEVES, AND OTHER ITEMS TO BE PLACED OR SET IN THE STRUCTURAL WORK. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COMPLYING WITH ALL SAFETY PRECAUTIONS AND REGULATIONS DURING THE WORK. THE CONTRACTOR SHALL BE RESPONSIBLE TO CONFIRM ALL THE FIELD DIMENSIONS ANY UNUSUAL CONSTRUCTION CONDITION THAT JEOPARDIZE SAFETY OF LABOR AND/OR PUBLIC, CONTRACTOR SHALL CONTACT ENGINEER IMMEDIATELY BEFORE PROGRESS, IN CASE OF AT THE TIME AND/OR IN FUTURE OR IN CASE OF THE ISCREPANCIES IN THE PROJECT

STRUCTURAL DESIGN SPECIFICATIONS

DESIGN CODE: 2020 Florida Building Code, 7th EDITION
AS APPLICABLE
WIND VELOCITY : 129 MPH PER ASCE 7-16 WIND MAP (STRENGTH LEVEL)

WIND EXPOSURE CATEGORY: B
SEISMIC DESIGN CATEGORY: E
ROOF LIVE LOAD: 20 PSF
DEAD LOADS: WEIGHT OF MATERIALS
LIVE LOAD: 40 PSF

SCOPE:

ANY ITEMS NOT SPECIFIED HEREIN SHALL FOLLOW THE REQUIREMENTS OF THE INTERNATIONAL RESIDENTIAL CODE'S PRESCRIPTIVE REQUIREMENTS. SUCH ITEMS MAY INCLUDE DETAILING OF FRAMING CONNECTIONS, SIZES OF MEMBERS, MATERIAL SPECIFICATIONS, AND OTHER REQUIREMENTS RELATED TO THE STRUCTURE. WHERE MANUFACTURED PRODUCTS ARE USED, THE DETAILING AS ESTABLISHED BY THE MANUFACTURER SHALL BE USED. THESE SPECIFICATIONS ARE BASED UPON CALCULATIONS FOR THE PROJECT. THE CALCULATIONS UTILIZE THE DOCUMENTS LISTED, AND ALL OF THE REFERENCED STANDARDS.

ROOF SHEATHING

THE ROOF SHALL BE SHEATHED WITH 5/8" THICK (MINIMUM) 24/16 APA-RATED PLYWOOD (OR OSB) WITH 0.131" DIAMETER X 2.5" LONG (RING SHANK) NAILS AT 3" ON CENTER AT ALL PANEL EDGES AND BOUNDARIES, AND 12" ON CENTER IN THE FIELD.

FLOOR SHEATHING

THE FLOOR SHALL BE SHEATHED WITH 3/4" THICK (MINIMUM) 24/16 APA-RATED PLYWOOD, WITH 0.131" DIAMETER X 2.5" LONG (RING SHANK) NAILS AT 3" ON CENTER AT ALL PANEL EDGES AND BOUNDARIES, AND 12" ON CENTER IN THE FIELD.

SHEARWALLS:

ALL CONTAINER WALLS ARE SHEARWALLS.

EARTHWORK

THE CONTRACTOR SHALL TAKE ALL NECESSARY MEASURES TO FULLY PROTECT ADJACENT PROPERTIES. THE WORK CONSISTS OF FURNISHING ALL LABOR, EQUIPMENT, MATERIALS, AND PERFORMING ALL OPERATIONS NECESSARY FOR ALL EARTHWORK, INCLUDING SECURING AND PAYING FOR THE GRADING PERMIT. EXCAVATIONS FOR FOOTINGS SHALL BE MADE TO THE WIDTH, LENGTH, AND DEPTH REQUIRED. FINISH WITH LEVEL BOTTOMS UNLESS SITE CONDITIONS REQUIRE A SLOPED BOTTOM. EXCAVATIONS SHALL BE KEPT FREE OF STANDING WATER. WHERE EXCAVATIONS ARE MADE TO DEPTH GREATER THAN INDICATED, SUCH ADDITIONAL DEPTH SHALL BE FILLED WITH CONCRETE AS SPECIFIED FOR FOOTINGS. FILL MATERIALS SHALL BE FREE FROM DEBRIS, VEGETABLE MATTER AND OTHER FOREIGN SUBSTANCES. ALL TRADES SHALL BACKFILL OWN TRENCHES, UNLESS OTHERWISE DIRECTED.

DESIGN PRESSURE

DESIGN PRESSURE FOR WINDOWS AND DOORS SHALL BE 35 PSF WITHIN 3' OF WALL CORNERS AND 30 PSF ALL OTHER LOCATIONS. PRESSURES PROVIDED AT SERVICE LEVEL (ASD).

CAST-IN-PLACE CONCRETE NOTES CONCRETE MIXES SHALL BE DESIGNATED PER ACI 301, USING PORTLAND CEMENT CONFORMING TO ASTM C-150 OR C-595, AGGREGATE CONFORMING TO ASTM C-33, AND ADMIXTURES CONFORMING TO ASTM C-494, C-1017, C818, AND C-260. CONCRETE SHALL BE READY MIXED IN ACCORDANCE WITH ASTM C-94.

CONCRETE SHALL CONFORM TO THE FOLLOWING, U.N.O.:

LOCATION	MIN f'c
FOUNDATION	2,500 PSI
SLAB ON GRADE	4,000 PSI

REINFORCING STEEL, INCLUDING HOOKS AND BENDS, SHALL BE DETAILED IN ACCORDANCE WITH ACI 315. ALL REINFORCING STEEL INDICATED AS BEING CONTINUOUS SHALL BE LAPPED WITH A TYPE 2 SPLICE UNLESS OTHERWISE NOTED. BAR SUPPORTS SHALL BE PROVIDED FOR ALL REINFORCING STEEL TO ENSURE MINIMUM CONCRETE COVER. BAR SUPPORTS SHALL BE PLASTIC TIPPED OR STAINLESS STEEL. CONCRETE EXPOSED TO WEATHER SHALL BE AIR ENTRAINED TO 5% (+-1%) WITH AN ADMIXTURE THAT CONFORMS TO ASTM C-260.

FOUNDATION NOTES

ALL FOOTINGS SHALL BEAR ON UNDISTURBED, FIRM NATURAL SOIL, OR COMPACTED FILL CAPABLE OF SUPPORTING A DESIGN BEARING PRESSURE OF 2000 PSF. CONTRACTOR SHALL CONFIRM BEARING CAPACITY WITH INDEPENDENT TESTING. REMOVE ANY SOFT OR FROZEN SOIL MATERIAL ENCOUNTERED UNDER FOOTINGS. UNLESS OTHERWISE NOTED, PROVIDE THE FOLLOWING COVER FOR FOUNDATION REINFORCEMENT: BOTTOM BARS & BARS IN CONCRETE CAST AGAINST EARTH: 3" BARS THAT ARE EXPOSED TO WEATHER:

#5 OR SMALLER 1 1/2"
#6 OR BIGGER 2"

ALL BARS SHALL BE LAPPED 40 X THE BAR DIAMETER AT SPLICES. PRIOR TO COMMENCING FOUNDATION WORK, COORDINATE WORK WITH UTILITIES.

A LAYER OF WELL-GRADED FREE-DRAINING GRANULAR MATERIAL/SAND (AT LEAST 6" THICK AND COMPACTED TO 98% OF SPMDD) SHOULD BE PLACED UNDER THE FOUNDATION TO PROVIDE A UNIFORM BEARING SURFACE.

STRUCTURAL STEEL NOTES

ALL DESIGN, FABRICATION AND ERECTION SHALL CONFORM TO THE AISC STEEL CONSTRUCTION MANUAL, 13TH EDITION, WELDING SHALL CONFORM TO THE LATEST AWS AND AISC SPECIFICATIONS.

WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE BEST PRACTICE AND WITHIN THE TOLERANCES SPECIFIED IN THE AISC SPECIFICATIONS FOR STRUCTURAL STEEL.

IT IS SPECIFICALLY NOTED THAT BURNED HOLES ARE NOT ACCEPTABLE UNLESS SPECIAL PERMISSION IS GIVEN BY ENGINEER.

ALL SHOP FABRICATED WORK SHALL BE DONE IN A SHOP APPROVED BY THE GOVERNING AGENCY. FABRICATOR SHALL SUBMIT PROGRAM OF WELDING INSPECTION TO ENGINEER FOR APPROVAL.

ALL STRUCTURAL STEEL SHALL BE AS FOLLOWS UNO:

ALL WF, WT SHAPES	A992 GRADE 50
CONNECTION PL & MISC STEEL (UNO)	ASTM A36
GUSSET & COLLECTOR PLATES	ASTM A572 GRADE 50
PIPE COLUMNS (TYPE S, SEAMLESS)	ASTM A53 GRADE B
STRUCTURAL TUBING	ASTM A500 GRADE B
ANGLE, CHANNELS	ASTM A36
THREADED ROD	ASTM A36
HEADED SHEAR STUDS	ASTM A108, GRADES 1015 TO 1020, TYPE 316, 50 ksi

ELECTRODES

a) E70XX FOR A36 STEEL AND SMAW PROCESS OR EQUIVALENT.

b) FOR OTHER STEEL GRADES USE MATCHING WELD METAL AND PROCESSES.

ALL HIGH STRENGTH BOLTS SHALL BE ASTM A325-N TYPE UNLESS OTHERWISE NOTED. ALL BOLTS USED FOR ERECTION SHALL BE ASTM A325 TYPE WITH THREADS EXCLUDED FROM SHEAR PLANES. ALL PLAIN ANCHORS SHALL BE A36; ALL ANCHOR BOLTS SHALL COMPLY WITH ASTM F1554. 3" MINIMUM CONCRETE COVER WILL BE PLACED AROUND ALL ANCHOR BOLTS EXPOSED TO THE WEATHER, U.N.O.

WELDING MATERIALS: PROVIDE TYPE REQUIRED FOR MATERIALS BEING WELDED, PER AWS D1.1.

PROVIDE CONTINUOUS INSPECTION FOR ALL FABRICATION AND WELDING OF STRUCTURAL STEEL AS REQUIRED PER CODE REQUIREMENTS. ALL COMPLETE PENETRATION GROOVE WELDS IN JOINTS AND SPLICES SHALL BE TESTED 100 PERCENT IN ACCORDANCE WITH IBC. USE ONE OF THE APPROVED METHODS OF TIGHTENING HIGH STRENGTH BOLTS.

A WELDING SEQUENCE SHALL BE PLANNED TO MINIMIZE RESIDUAL STRESSES AND DISTORTIONS OF INDIVIDUAL MEMBERS AND THE BUILDING FRAME. ALL DETAILING, FABRICATION, AND ERECTION SHALL COMPLY WITH AISC, LATEST EDITION.

UNLESS OTHERWISE NOTED, ALL STIFFENER PLATES ARE 3/8" THICK MINIMUM AND ALL BUTT WELDS ARE FULL PENETRATION WELDS. ERECTION CLIPS, TEMPORARY BRACING, ETC., REQUIRED BY THE CONTRACTOR ARE NOT SHOWN.

SUBMIT SHOP DRAWINGS FOR THE FABRICATION AND ERECTION OF ALL ASSEMBLIES OF STRUCTURAL STEEL WORK. INCLUDE PLANS AND ELEVATIONS AT NOT LESS THAN 1/4" TO 1'-0" SCALE, AND INCLUDE DETAILS OF SECTIONS AT NOT LESS THAN 1" TO 1'-0" SCALE.

NO FINISH FABRICATION SHALL BE COMMENCED OR MATERIAL DELIVERED TO THE JOB UNTIL THE ENGINEER HAS REVIEWED AND APPROVED THE SHOP DRAWINGS.

ALL STRUCTURAL STEEL SHALL BE PAINTED WITH ONE SHOP COAT OF ZINC CHROMATE PRIMER OR EQUAL. AFTER ERECTION, FIELD CONNECTIONS SHALL BE TOUCHED UP. DO NOT PAINT PORTION OF STEEL TO BE EMBEDDED IN CONCRETE, HEADED ANCHOR STUDS, FAYING SURFACES OR AREAS TO RECEIVE FIRE PROOFING. EXTERIOR, EXPOSED STEEL MEMBERS ARE SPECIFIED TO BE HOT-DIPPED GALVANIZED OR STAINLESS AS NOTED.

WELD LENGTHS CALLED FOR ON PLANS ARE THE NET EFFECTIVE LENGTH REQUIRED. WHERE FILLET WELD SYMBOL IS GIVEN WITHOUT INDICATION OF SIZE, USE MINIMUM SIZE WELDS AS SPECIFIED IN AISC MANUAL OF STEEL CONSTRUCTION LATEST EDITION. THIS INCLUDES OPEN WEB JOIST CONNECTIONS. THE USE OF E70T-4 WELDING WIRE IS NOT ALLOWED FOR ANY APPLICATION.

SHIPPING CONTAINERS CONSTRUCTION

SHIPPING CONTAINERS CONSTRUCTION SHALL MEET AC462 AND APPLICABLE STRUCTURAL BUILDING MATERIALS INCLUDING HIGH-STRENGTH WEATHERING STEEL (CORTEN) STRUCTURAL MATERIAL PROPERTIES FOR EXISTING SHIPPING CONTAINER STEEL COMPONENTS SHALL BE ESTABLISHED BY MATERIAL TESTING WHERE THE STEEL GRADE AND COMPOSITION CANNOT BE IDENTIFIED BY THE MANUFACTURER'S DESIGNATION AS TO MANUFACTURE AND MILL TEST.

STRUCTURAL STEEL NOTES (CONT.)

WRITTEN WELDING PROCEDURE SPECIFICATIONS (WPS) PER THE RECOMMENDATIONS OF THE AMERICAN WELDING SOCIETY (AWS) SHALL BE DEVELOPED BY THE FABRICATOR/ERECTOR AND SUBMITTED FOR REVIEW BY THE ENGINEER PRIOR TO ANY WELDING OF THE STRUCTURAL STEEL. THE WELDING PROCEDURES SHALL INCLUDE ALL THE WELDED JOINTS AND CONFIGURATIONS TO BE USED ON THIS PROJECT-ONLY WPS WHICH ARE RELEVANT TO THIS PROJECT SHALL BE SUBMITTED. ALL WELDED JOINTS SHALL BE PRE-QUALIFIED PER AWS OR BE QUALIFIED BY TEST PER AWS. A PROCEDURE QUALIFICATION RECORD (PQR) SHALL BE INCLUDED WITH THE WPS IF THE WELDING PROCEDURE OR JOINT IS QUALIFIED BY TESTING. THE ELECTRODE MANUFACTURER AND PRODUCT/TRADE NAME SHALL BE IDENTIFIED IN THE WPS IN ADDITION TO THE AWS ELECTRODE CLASSIFICATION NAME. A COPY OF THE ELECTRODE MANUFACTURER'S TECHNICAL DATA SHEETS WITH THE RECOMMENDED WELDING PARAMETERS SHALL BE SUBMITTED WITH THE WPS.

DO NOT WELD ANY STRUCTURAL STEEL MEMBER OR CONNECTION UNLESS EXPLICITLY CALLED OUT IN THE CONTRACT DOCUMENTS.

WELD SYMBOLS SHOW FINAL WELD REQUIRED. THE CHOICE TO WELD IN THE FIELD OR IN THE SHOP SHALL BE UP TO THE CONTRACTOR AND SHALL BE INDICATED IN THE FABRICATOR'S SHOP DRAWINGS.

ALL STRUCTURAL STEEL SHALL BE PROPERLY GUYED AND BRACED UNTIL FLOOR AND ROOF FRAMING SYSTEM AND LATERAL LOAD RESISTING SYSTEM IS IN PLACE.

THIS STRUCTURE IS CONSIDERED A NON-SELF-SUPPORTING STEEL FRAME. THE CONTRACTOR SHALL PROVIDE ADEQUATE TEMPORARY SUPPORTS UNTIL ALL PERMANENT SHEAR WALLS, MOMENT FRAMES, BRACED FRAMES, AND FLOOR SLABS ARE IN PLACE.

ALL BEAM CONNECTIONS SHALL BE DETAILED TO PROVIDE A SHEAR CONNECTION WITH A MINIMUM DESIGN CAPACITY AS THAT SHOWN IN THE DRAWINGS BEAM SHEAR TAB CONNECTION TABLE FOR THE CORRESPONDING BEAM SIZE.

ALL CONNECTIONS NOT DETAILED OR OTHERWISE NOTED SHALL BE DESIGNED AS AISC TYPE 2 BOLTED CONNECTIONS DESIGNED FOR FULL LOAD CAPACITY OF THE CONNECTING MEMBERS.

ALL BOLTS IN MOMENT CONNECTIONS SHALL BE BEARING TYPE CONNECTIONS.

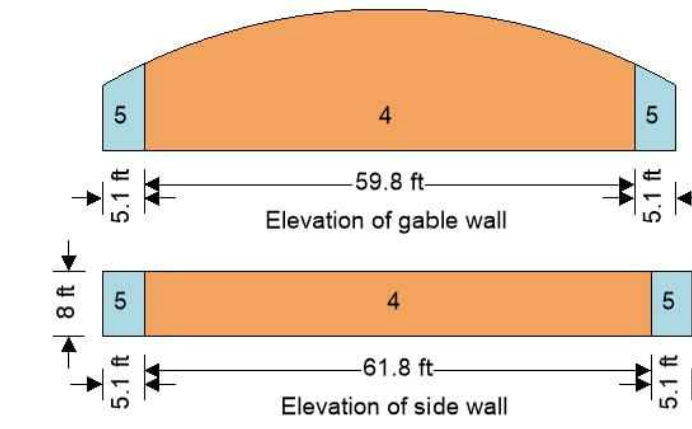
ALL WELDS SHALL BE MADE BY WELDERS CERTIFIED ACCORDING TO AWS PROCEDURES.

NOTE:

- * ALL DIMENSIONS TO BE VERIFIED IN FIELD.
- * ALL EXISTING DIMENSIONS SUBMITTED BY THE CLIENT.
- * FASTENING REQUIREMENTS FOR WOOD TOGETHER TO BE ACCORDING TO THE MANUFACTURE RECOMMENDATIONS.

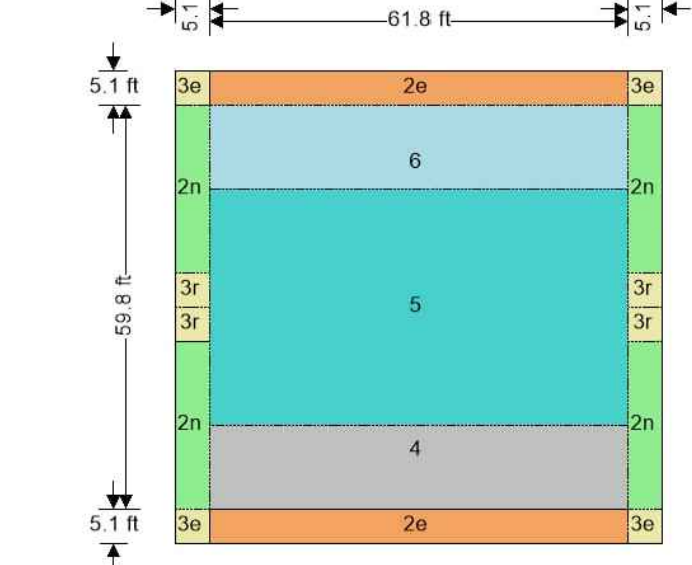
Components and cladding pressures - Wall (Table 30.3-1)

Component	Zone	Length (ft)	Width (ft)	Eff. area (ft²)	+GC _w	-GC _w	Pres (+ve) (psf)	Pres (-ve) (psf)
<=10 sf	4	-	-	10.0	0.90	-0.99	22.3	-24.1
50 sf	4	-	-	50.0	0.79	-0.88	20.0	-21.9
200 sf	4	-	-	200.0	0.69	-0.78	18.0	-19.9
>500 sf	4	-	-	500.1	0.63	-0.72	16.7	-18.6
<=10 sf	5	-	-	10.0	0.90	-1.26	22.3	-29.7
50 sf	5	-	-	50.0	0.79	-1.04	20.0	-25.1
200 sf	5	-	-	200.0	0.69	-0.85	18.0	-21.2
>500 sf	5	-	-	500.1	0.63	-0.72	16.7	-18.6



Components and cladding pressures - Roof (Figure 27.3-3 and Figure 30.3-2D)

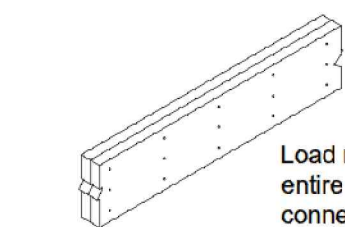
Component	Zone	Length (ft)	Width (ft)	Eff. area (ft²)	+GC _r	-GC _r	Pres (+ve) (psf)	Pres (-ve) (psf)
>100 sf	2e	-	-	100.1	0.80	-0.80	20.2	-20.2
<=10 sf	2n	-	-	10.0	0.90	-2.00	22.3	-45.0
50 sf	2n	-	-	50.0	0.83	-1.46	20.8	-33.9
100 sf	2n	-	-	100.0	0.80	-1.23	20.2	-29.1
>200 sf	2n	-	-	200.1	0.80	-1.00	20.2	-24.4
100 sf	3e	-	-	100.0	0.80	-1.48	20.2	-34.3
>300 sf	3e	-	-	300.1	0.80	-1.00	20.2	-24.4
100 sf	3r	-	-	100.0	0.80	-1.23	20.2	-29.1
>200 sf	3r	-	-	200.1	0.80	-1.00	20.2	-24.4
50 sf	4	-	-	50.0	0.83	-1.08	20.8	-26.0
>100 sf	4	-	-	100.1	0.80	-1.08	20.2	-26.0
50 sf	5	-	-	50.0	0.83	-1.00	20.8	-24.4
>100 sf	5	-	-	100.1	0.80	-1.00	20.2	-24.4
50 sf	6	-	-	50.0	0.83	-0.60	20.8	-16.1
>100 sf	6	-	-	100.1	0.80	-0.60	20.2	-16.1



Fastener Installation Requirements

Piece Width	# of Piles	Fastener				Location
		Type ⁽¹⁾	Min. Length	# Rows	O.C. Spacing	
1 3/4"	2	10d nails	3"	3 ⁽²⁾	12"	One side
		12d-16d nails	3 1/4"	2 ⁽²⁾	12"	
	3	Screws	3 3/8" or 3 1/2"	2	24"	Both sides
		10d nails	3"	3 ⁽²⁾	12"	
3 1/2"	4	12d-16d nails	3 1/2"	2 ⁽²⁾	12"	One side (per ply)
		Screws	5"	2	24"	
	2	10d nails ⁽³⁾	3"	3 ⁽²⁾	12"	Both sides
		12d-16d nails ⁽³⁾	3 1/2"	2 ⁽²⁾	12"	
3 1/2"	2	Screws	5" or 6"	2	24"	One side
		1/2" bolts	6 3/4"	2	24"	

When fasteners are required on both sides, stagger fasteners on the second side so they fall halfway between fasteners on the first side.




Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.



Multiple pieces can be nailed or bolted together to form a header or beam of the required size, up to a maximum width of 7"

(1) 10d nails are 0.128" diameter; 12d-16d nails are 0.148" - 0.162" diameter; screws are SDS, SDW, WS, or Truss-LOK-EWP™.
(2) An additional row of nails is required with depths of 14" or greater.
(3) When connecting 4-ply members, nail each ply to the other and offset nail rows by 2" from the rows in the ply below.



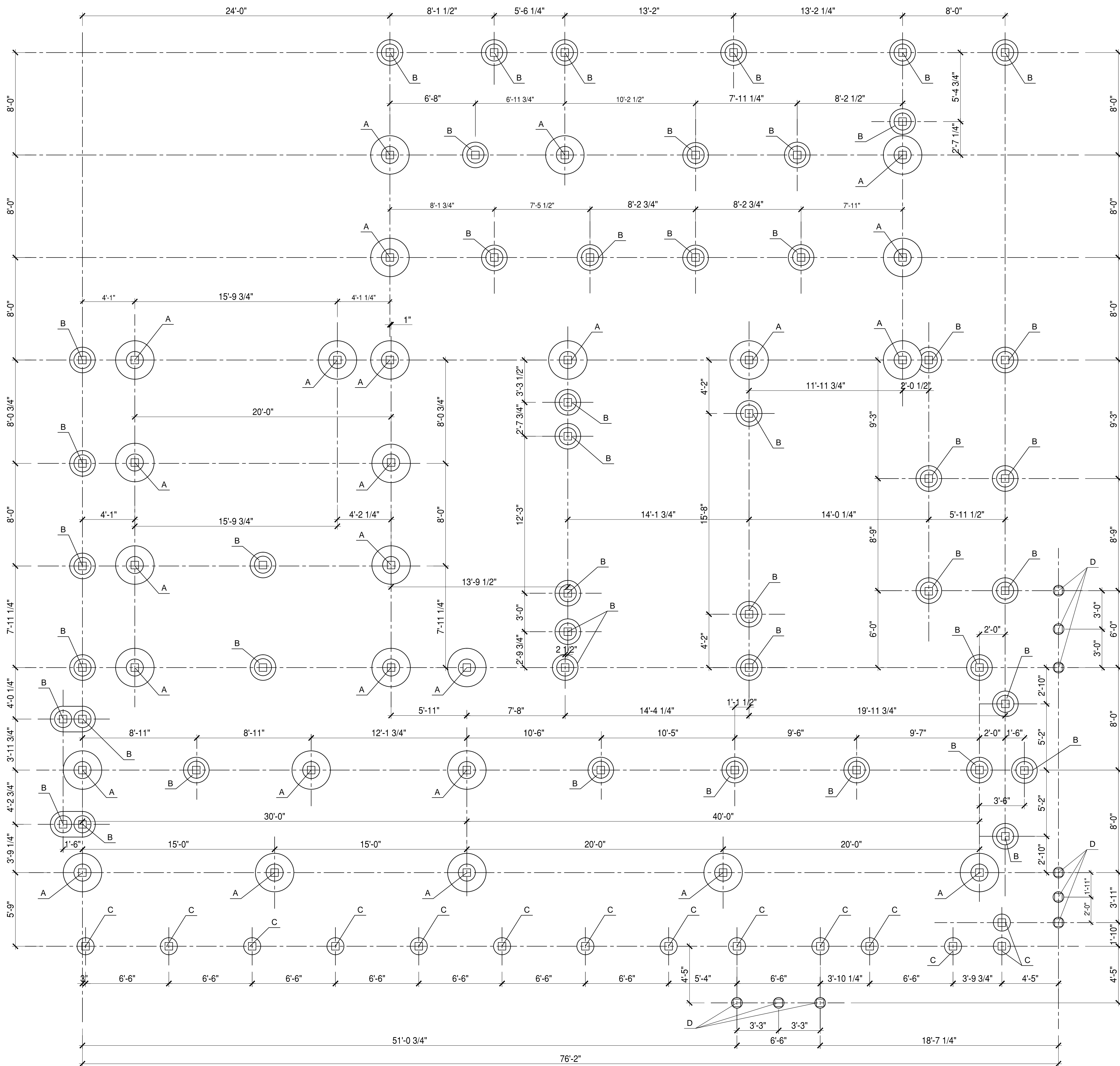
www.kiroengineering.com
KINGS PARK, NEW YORK 11754
TEL : 646 558 1332, FAX : 646 558 1338

GRANT RESIDENCE
108 3RD STREET
ST. AUGUSTINE, FL 32084

JOB # 2022 DATE 9/16/22
DRAWN BY: V.H.
DESIGNED BY: M.M.
SHEET # 5

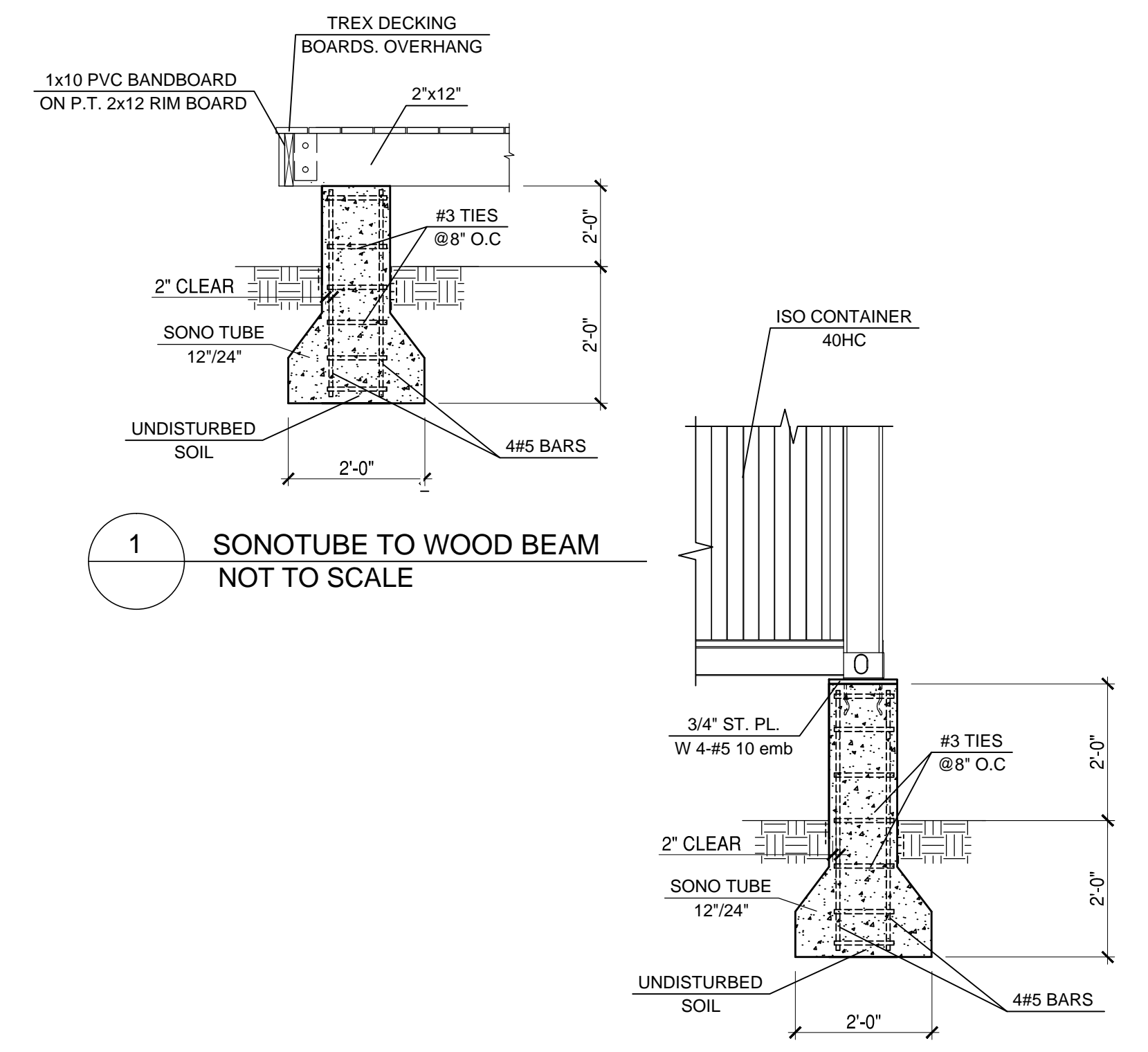
G 1
SHEET TITLE: GENERAL NOTES

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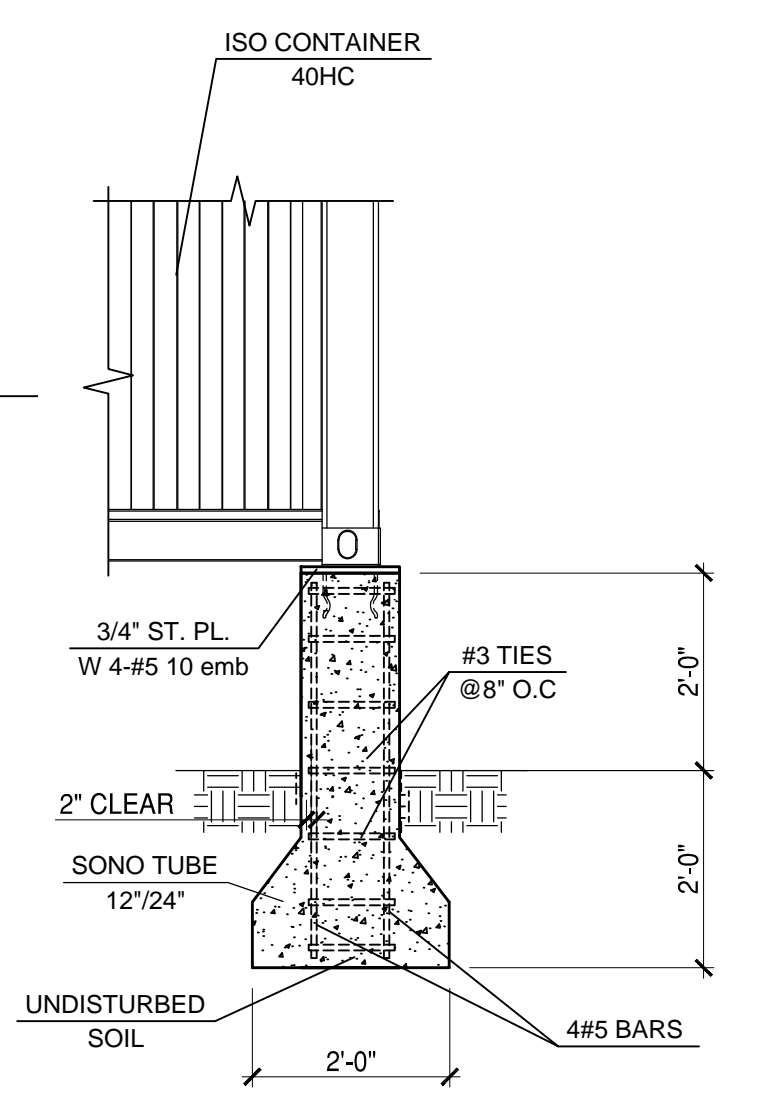


1 SONOTUBE PLAN
SCALE: 1/4" = 1'-0"

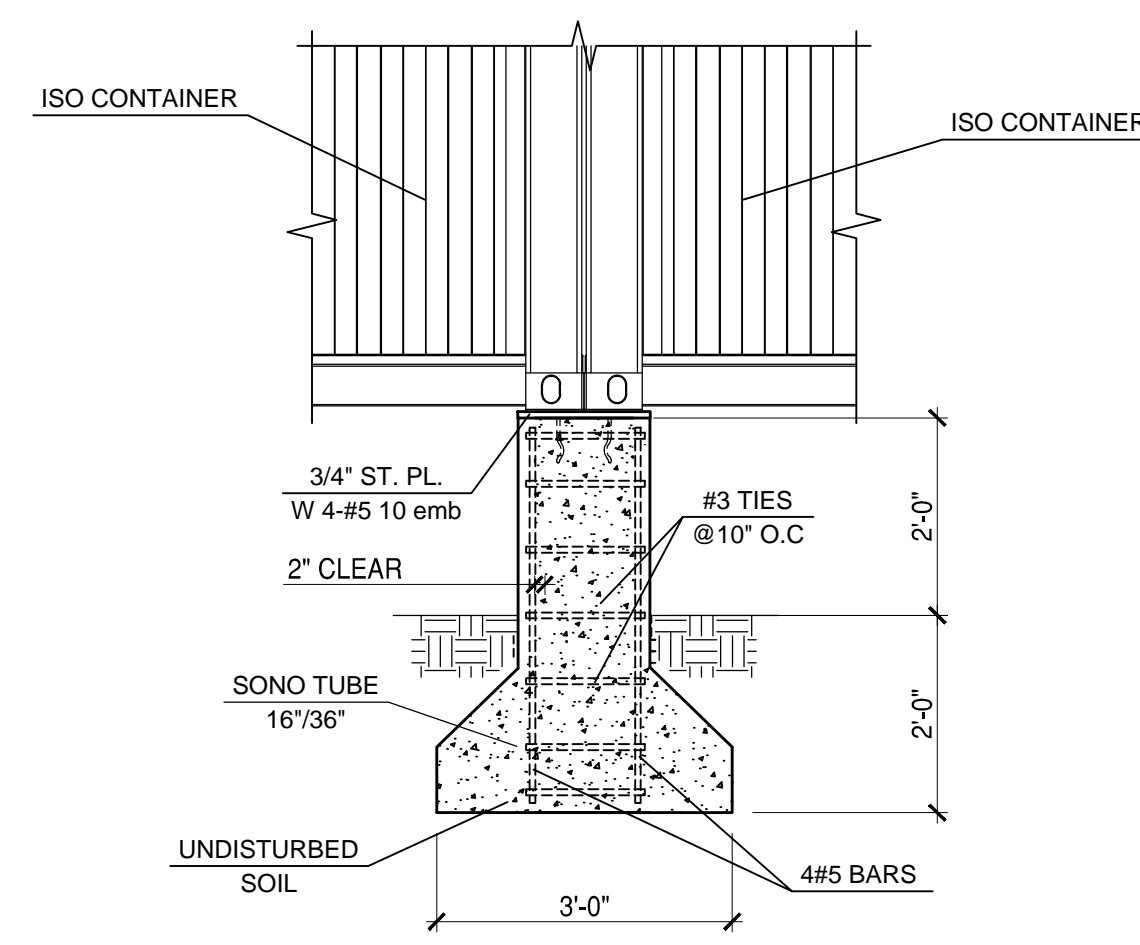
SONOTUBES LOADS/LRFD				
TYPE	Q ty	DEPTH	DIAM	MAX Compr.
A	26	48"	16"/36"	
B	47	48"	16"/24"	
C	13	48"	16"	
D	9	36"	10"	



1 SONOTUBE TO WOOD BEAM
NOT TO SCALE



3 SONOTUBE TO CONTAINER
NOT TO SCALE



2 SONOTUBE TO CONTAINER
NOT TO SCALE

FOUNDATION NOTES:

1. ALL SLABS AND CONC. BEAMS SHALL HAVE A 6 MIL GEO-TEXTILE UNDERLAY TYP.
 2. CONCRETE TO BE ACI 301-66, TYPE II CEMENT, 3500 MIN PSI AT 28 DAYS, 5" MAXIMUM SLUMP.
 3. REINFORCING TO BE ASTM A615-BARS WITH F_y=60 KSI LAMP, 30 DIAMETER MINIMUM AT SPLICES OR WELD PER ACI STD.
 4. ALL ANCHOR BOLTS SHALL BE A-307 EMBEDDED 6" MINIMUM INTO CONCRETE OR MASONRY GROUT AND SPACED NOT MORE THAN 6 FEET APART. CONTRACTOR TO OPENING.
 5. PROVIDE 3/4" BEVELS AT ALL CORNERS OF EXPOSED CONCRETE.
 6. MAXIMUM LENGTH OF SLAB POURS BETWEEN CONSTRUCTION JOINTS SHALL NOT EXCEED 120 FEET.
 7. MINIMUM BAR EDGE CLEARANCE SHALL BE 2"
 8. STUB UP ALL 2" DRAIN PIPES.
- TAR SEAL AROUND PIPE AT PENETRATIONS TYP.

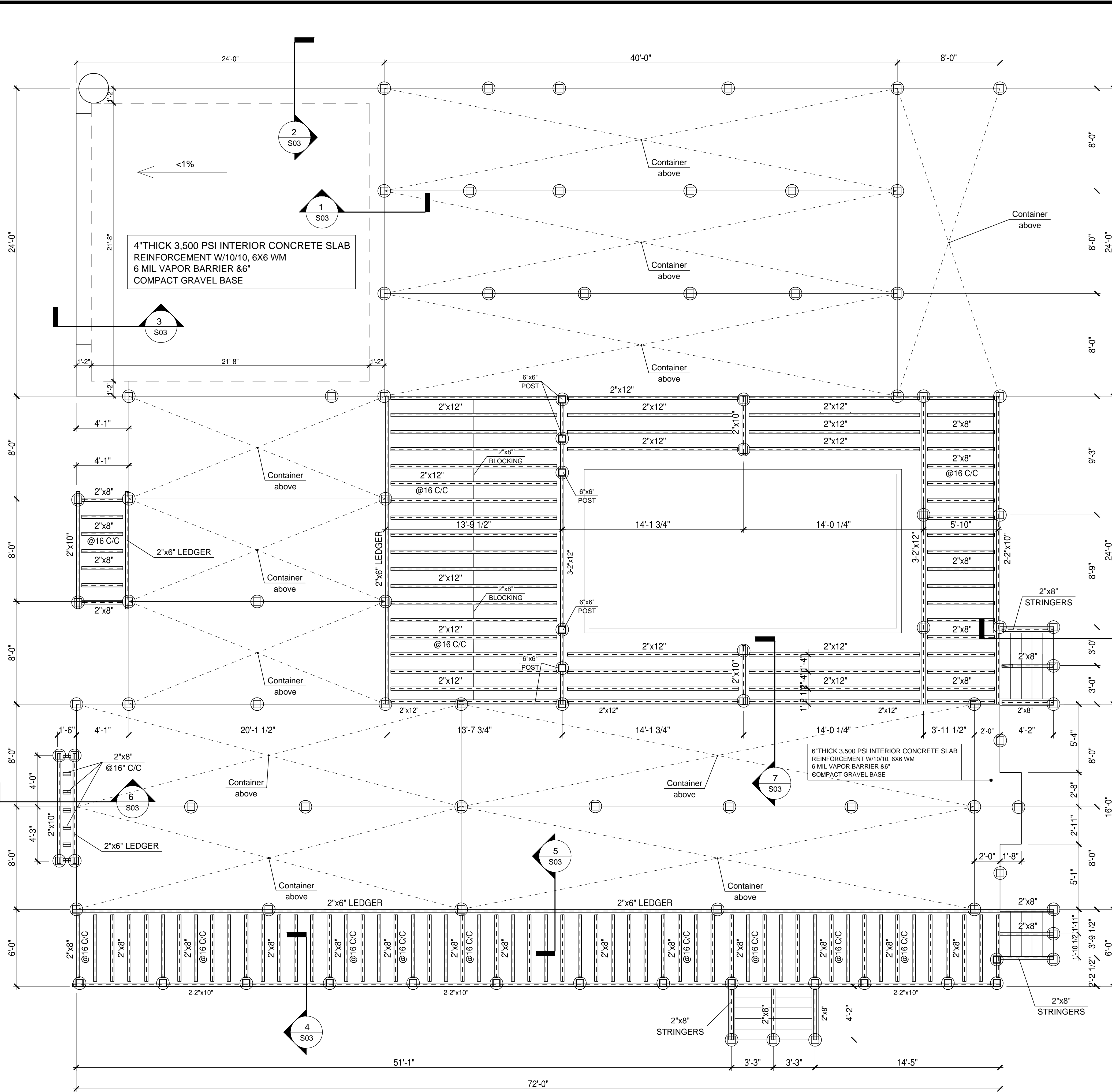


Kiro Engineering
www.kiroengineering.com
KINGS PARK, NEW YORK 11754
TEL : 646 558 1332, FAX : 646 558 1338

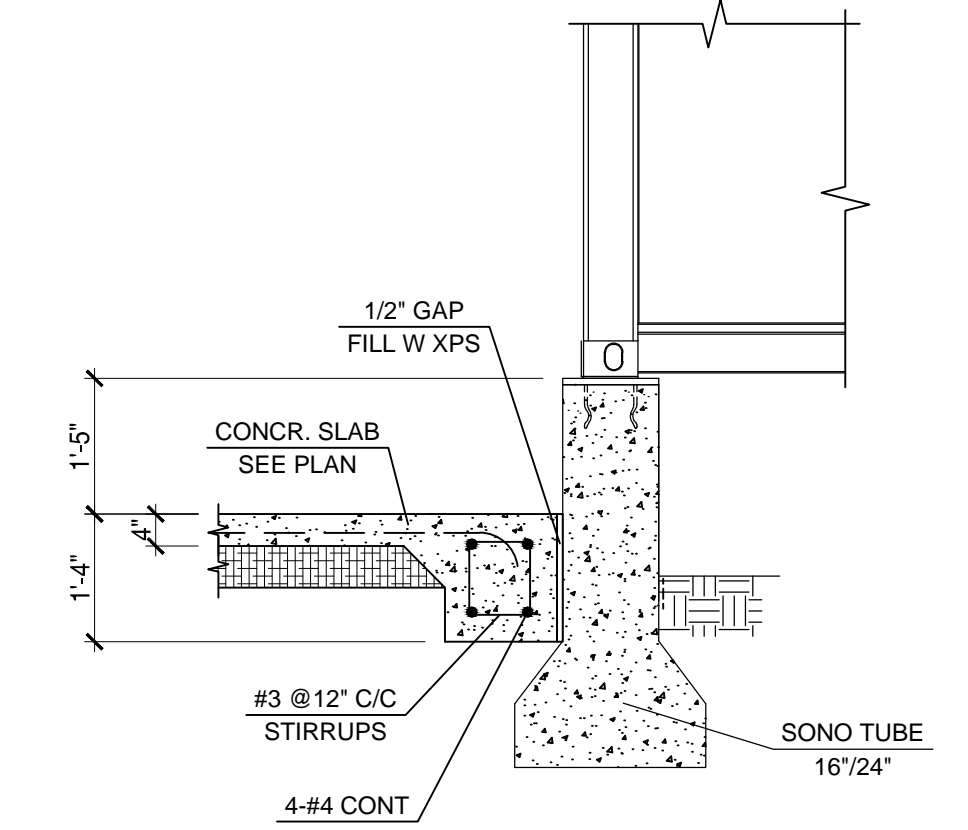
GRANT RESIDENCE
108 3RD STREET
ST. AUGUSTINE, FL 32084

JOB #	2022	DATE	9/16/22
DRAWN BY:	V.H.	DESIGNED BY:	M.M.
SHEET # 5	S 2		
SHEET TITLE: ISBU LAYOUT PLAN			

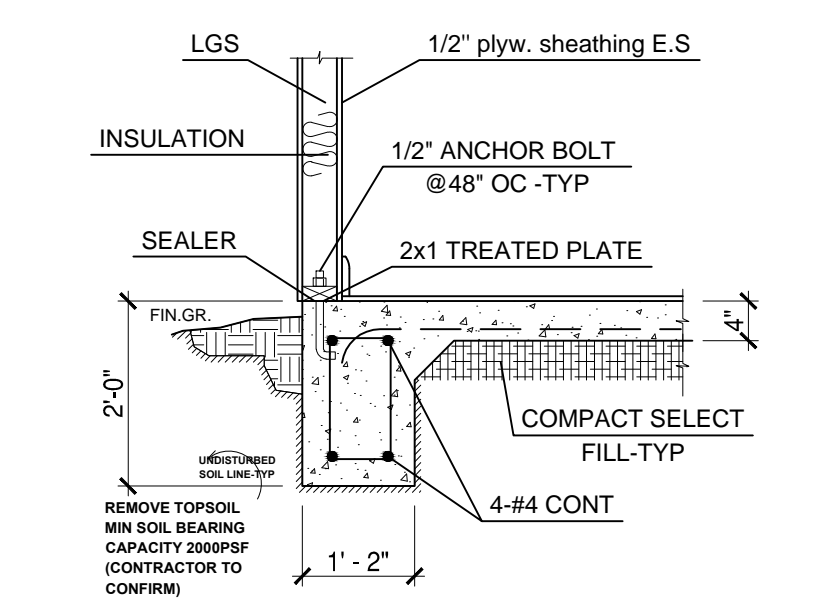
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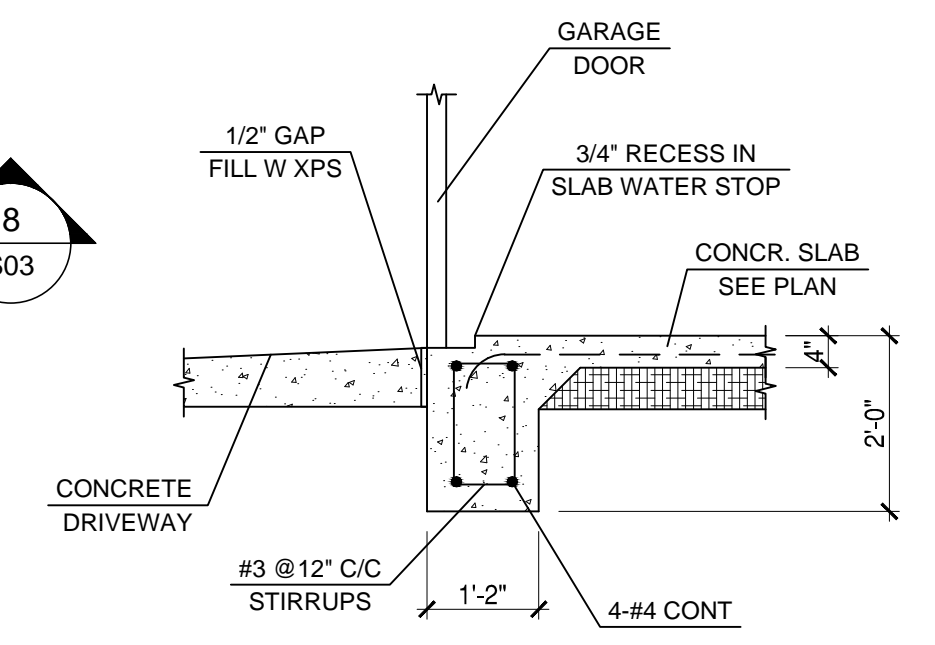
1 DECK JOISTS PLAN
SCALE: 1/4" = 1'-0"



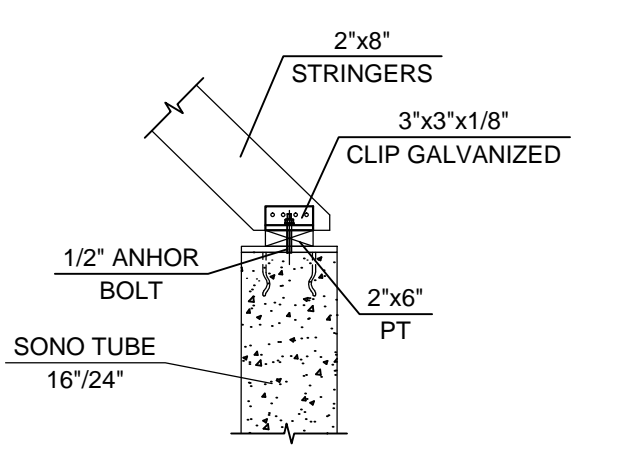
1 FOUNDATION DETAILS
NOT TO SCALE



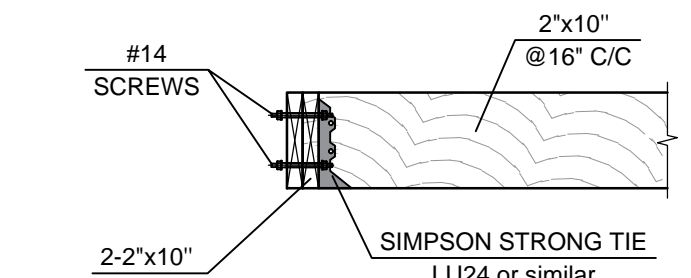
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NOT TO SCALE



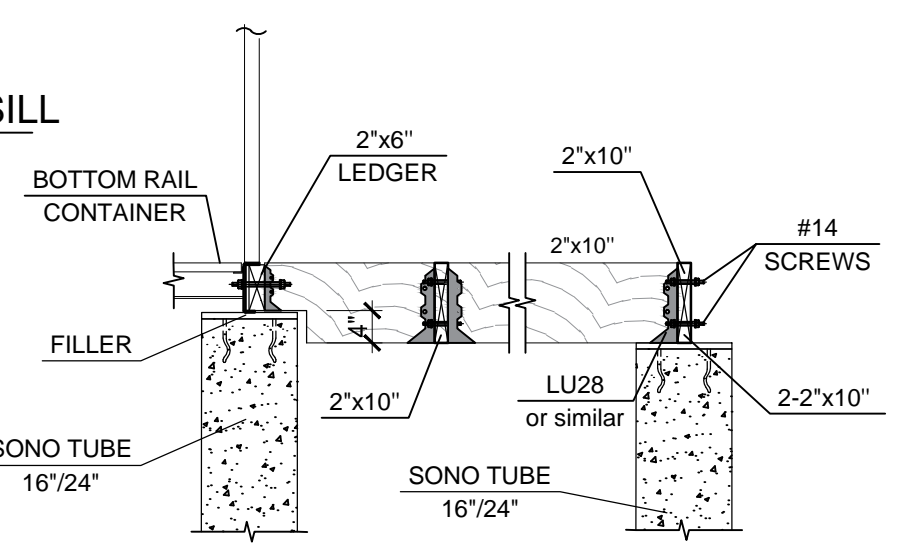
3 GARAGE DOOR RECESSED SILL
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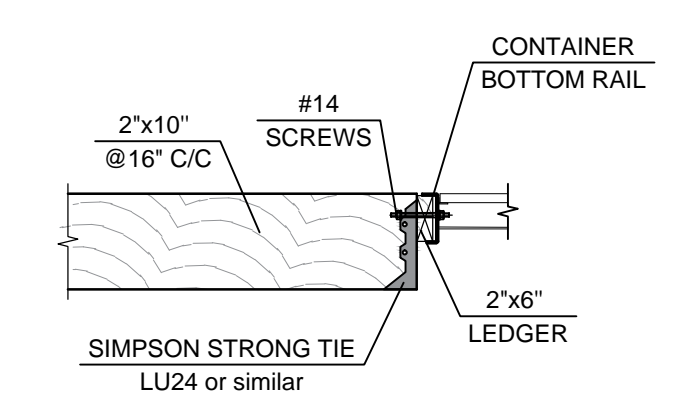
8 WOOD BEAM DETAIL
NOT TO SCALE



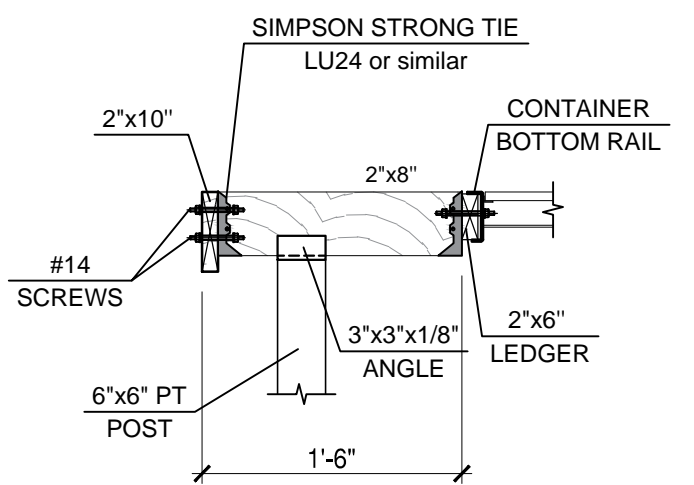
4 LEDGER BOARD DETAIL
NOT TO SCALE



7 WOOD BEAM DETAIL
NOT TO SCALE



5 LEDGER BOARD ATTACHEMENT DETAIL
NOT TO SCALE



6 WOOD BEAM DETAIL
NOT TO SCALE

EHM

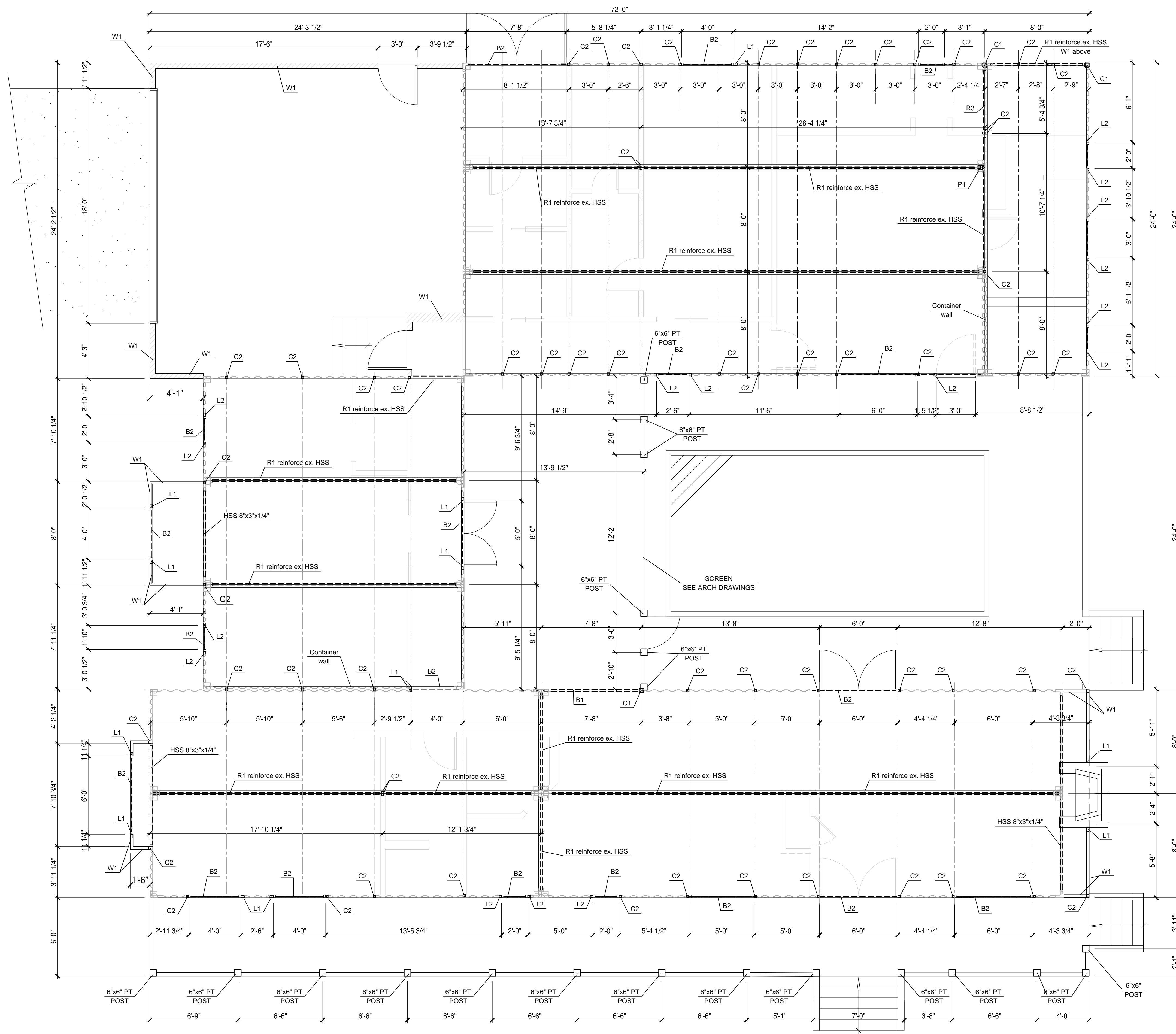
Kiro Engineering
www.kiroengineering.com

KINGS PARK, NEW YORK 11754
TEL : 646 558 1332, FAX : 646 558 1338

GRANT RESIDENCE
108 3RD STREET
ST. AUGUSTINE, FL 32084

JOB #	2022	DATE	9/16/22
DRAWN BY:	V.H.	DESIGNED BY:	M.M.
SHEET # 5	S 3		
SHEET TITLE: DECK JOISTS PLAN			

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1
S4
GROUND FLOOR PLAN
SCALE: 1/4" = 1'-0"

STEEL ELEMENTS SCHEDULE

Mark	PROFILE	GRADE
C1	HSS 4"x4"x $\frac{3}{4}$ "	A53 Gr.B
C2	HSS 3"x3"x $\frac{3}{4}$ "	A53 Gr.B
L1	HSS 2"x4"x $\frac{3}{4}$ "	A53 Gr.B
L2	HSS 2"x2"x $\frac{3}{4}$ "	A53 Gr.B
B1	HSS 4"x4"x $\frac{3}{8}$ "	A53 Gr.B
B2	HSS 4"x2"x $\frac{3}{8}$ "	A53 Gr.B
R1	HSS 2"x2"x $\frac{3}{8}$ "	A53 Gr.B
P1	EXISTING CONTAINER POST	
R2	HSS 4"x4"x $\frac{1}{4}$ "	A53 Gr.B
R3	W6x21	

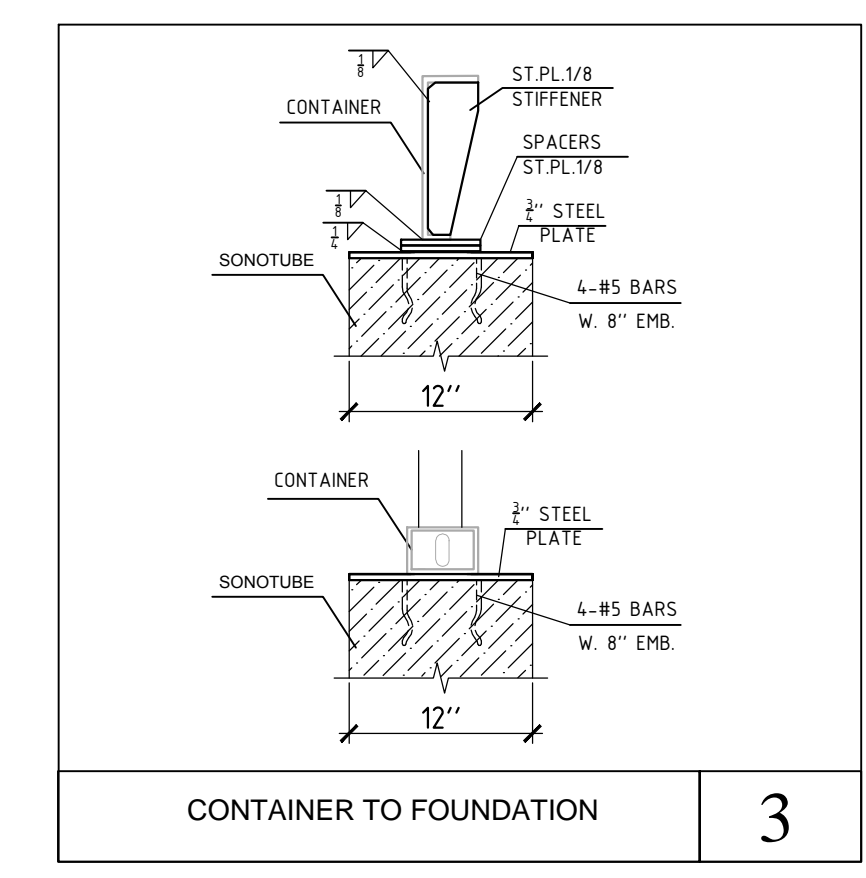
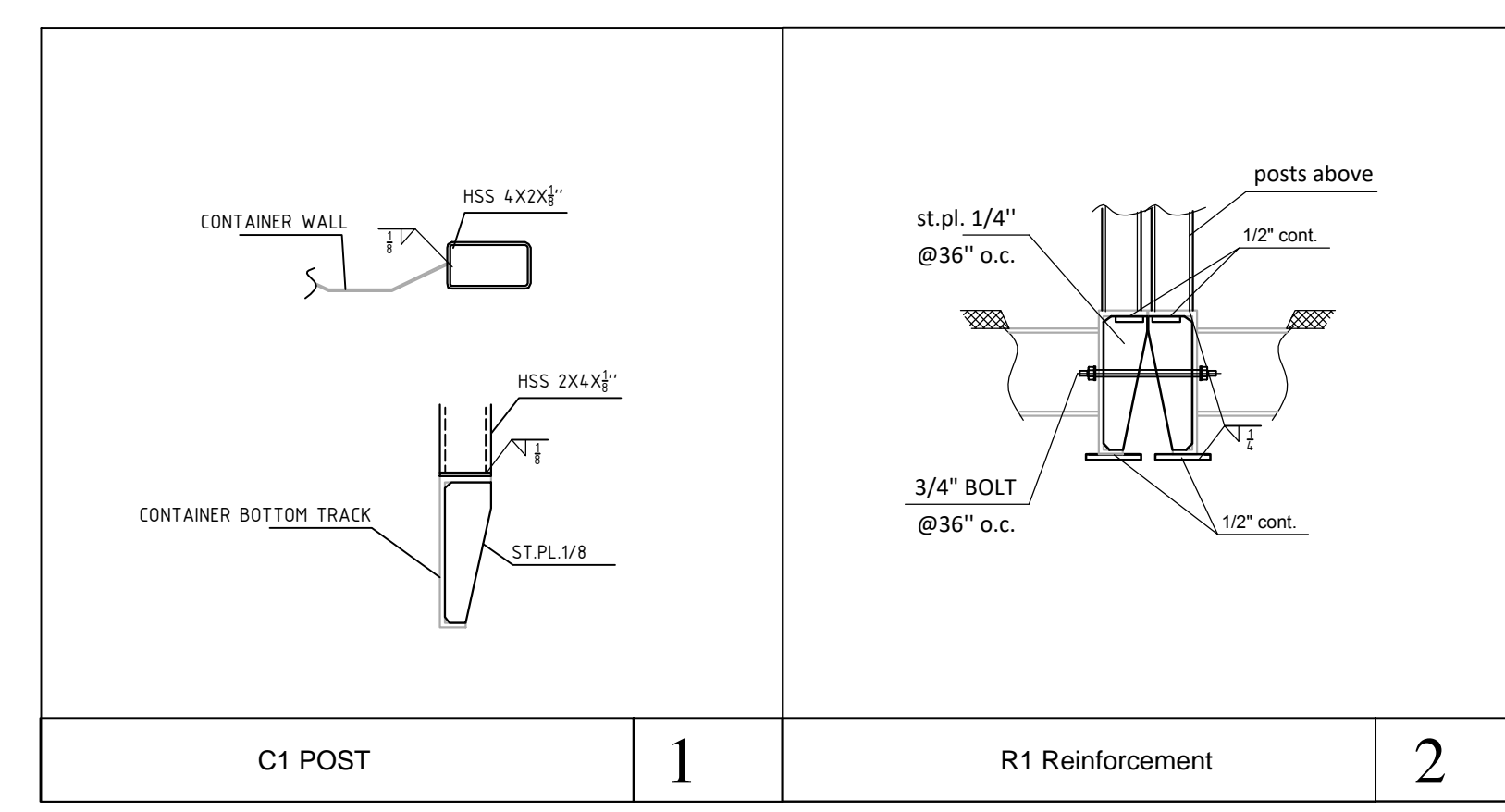
BEARING WALLS SCHEDULE

Mark	USE	STUDS	NOTE
W1	EXTERIOR	600S200-54 @ 16 C.C.	1/2" plyw. sheathing E.S
W2	INSIDE	600S200-54 @ 16 C.C.	1/2" plyw. sheathing E.S

PROVIDE 150U50-54(33) CRC BRACING @ 4' O.C. IN ALL WALLS ALIGN JOISTS WITH STUDS BELOW
Screws for W1, W2 6" o.c. @ all edges 12" o.c. @ field

FLOOR@ROOF JOISTS SCHEDULE

Mark	USE	JOISTS	TRACKS
RJ1	ROOF	2"x8" @ 24" O/C	



- Field welding shall be performed by a welder certified for AWS D1.1 structural steel in accordance with approved welding procedures by CWI
- Screws shall be ITW Buildex or approved equal.
- Face grain of plywood shall be perpendicular to supports.
- All steel wide flange columns and beams shall be A992 or A572 (Gr. 50). Contractor and/or Steel fabricator shall field verify all locations and elevations of steel members prior to fabrication to ensure a proper fit.
- Floor Sheathing shall be 1 1/8" (2-4-1) tongue and groove exterior APA plywood. Plywood shall be manufactured with exterior glue. Plywood sheathing shall be glued to steel framing members and #8-24 plymetal screws at 2.5" o.c. at all boundary, 4" at other edges and supported edges 10" o.c. at intermediate supports. Diaphragm edge blocking shall be required.

EHM

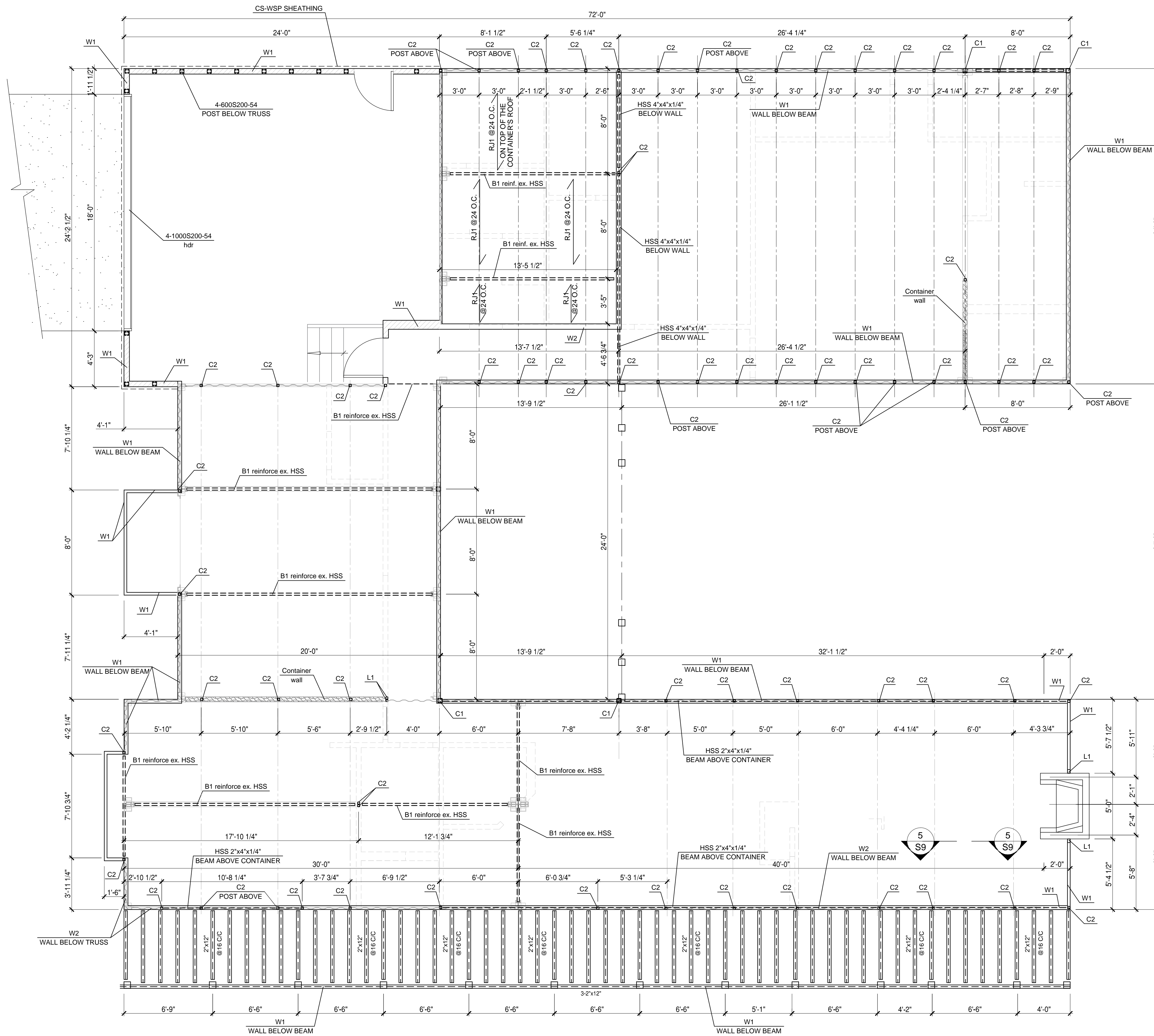
Kirol Engineering LLC
www.kirolengineering.com

KINGS PARK, NEW YORK 11754
TEL : 646 558 1332, FAX : 646 558 1338

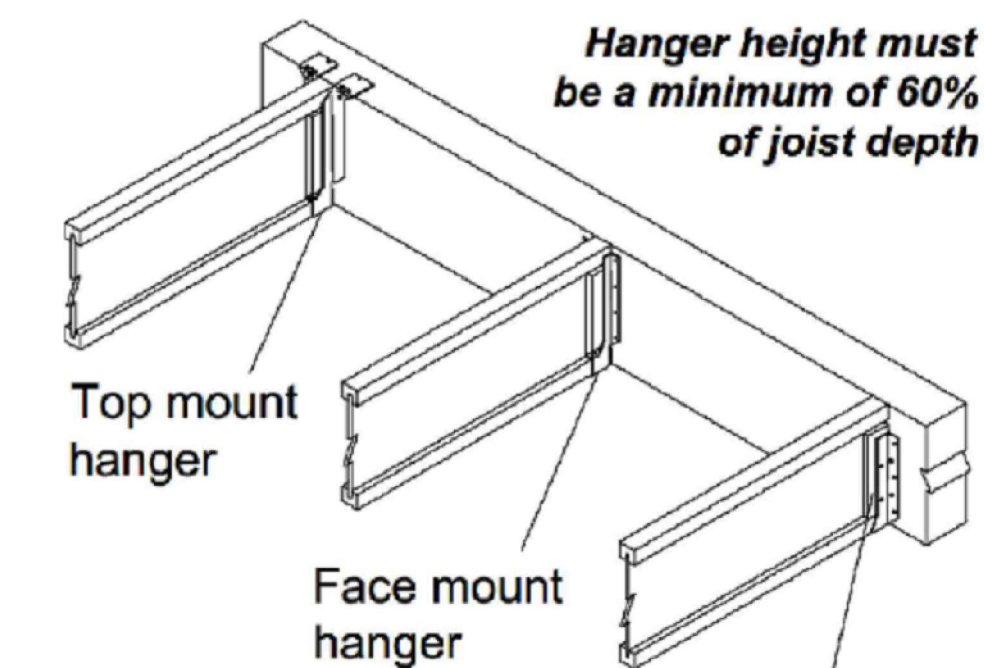
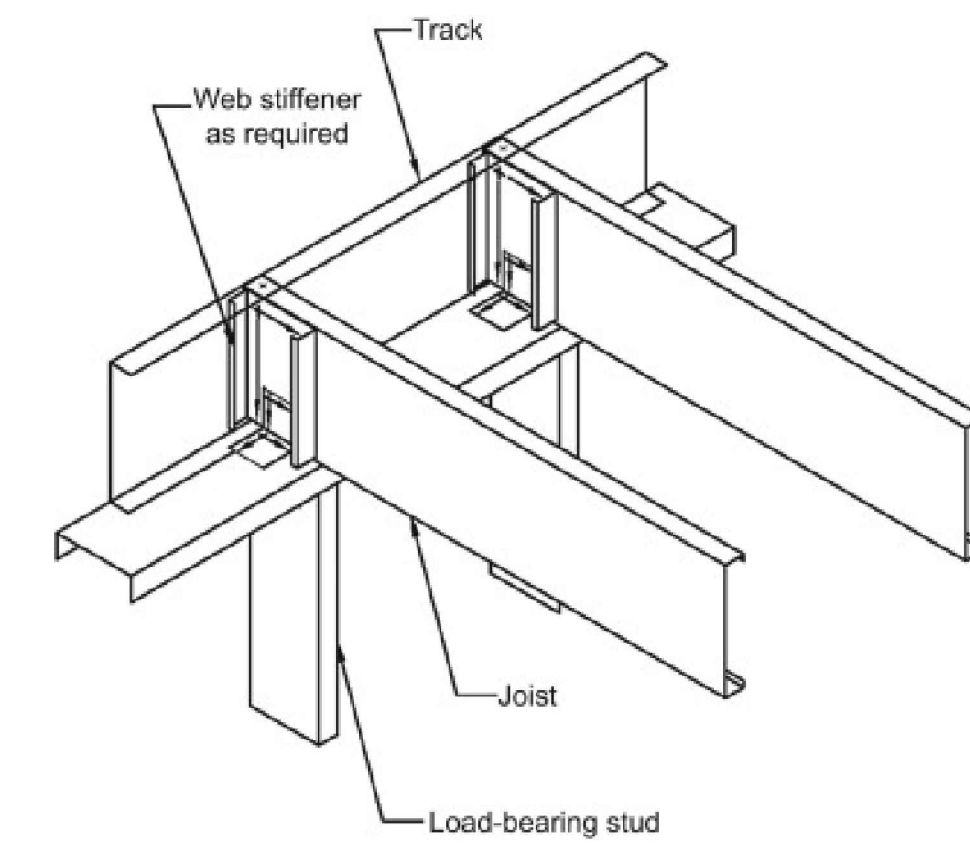
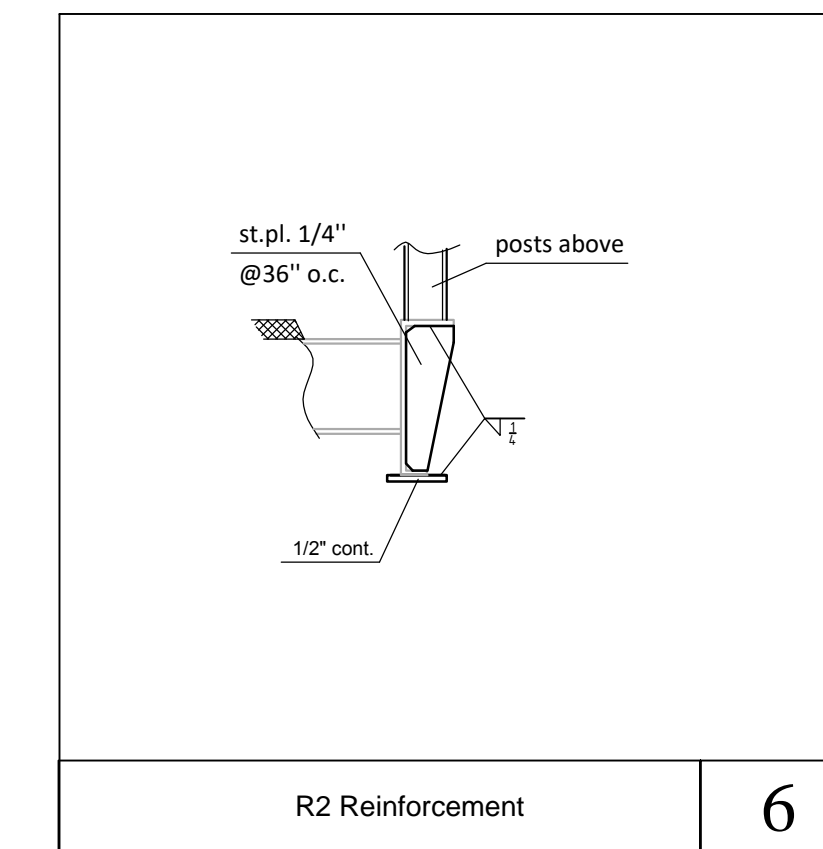
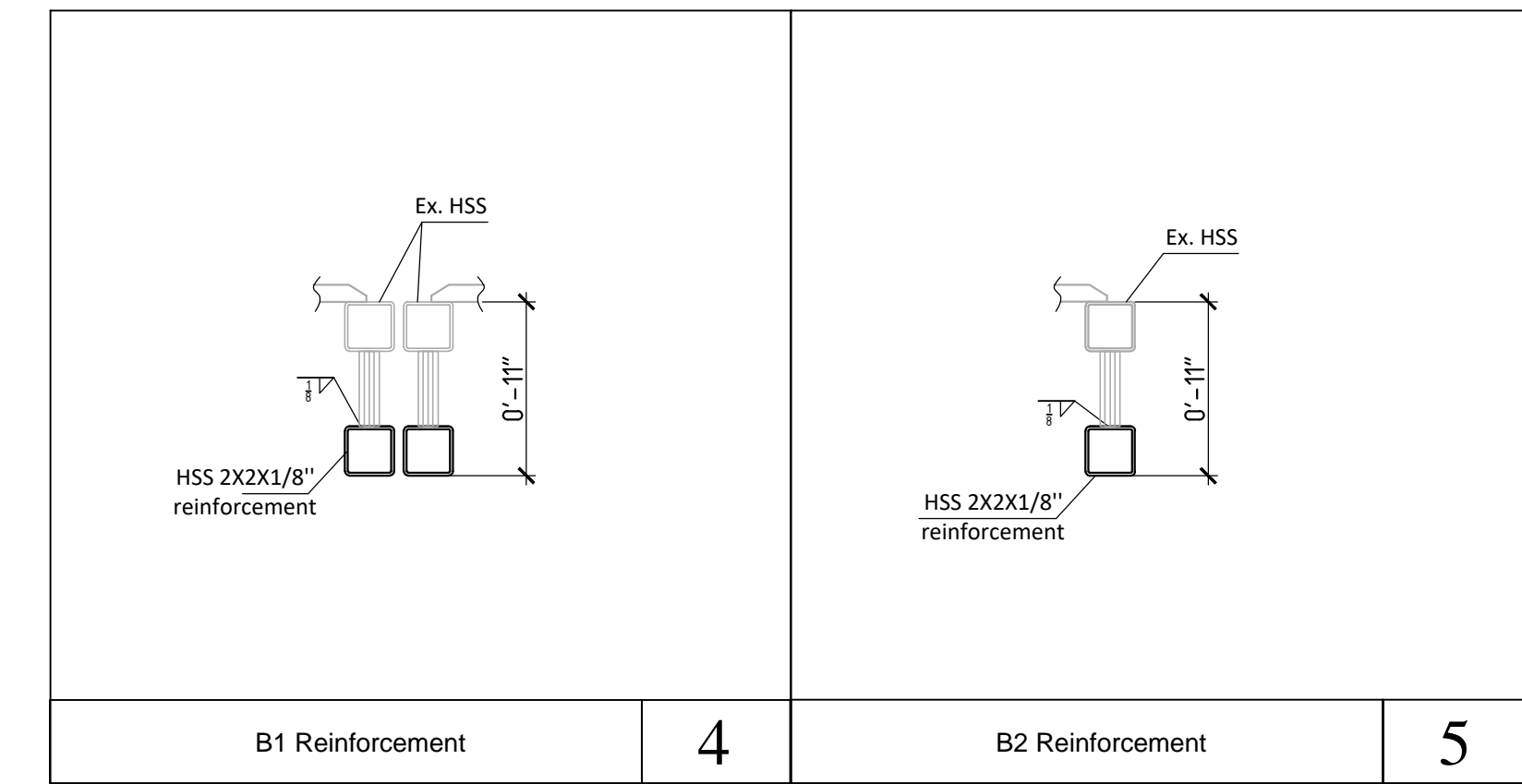
GRANT RESIDENCE
108 3RD STREET
ST. AUGUSTINE, FL 32084

JOB #	2022	DATE	9/16/22
DRAWN BY:	V.H.		
DESIGNED BY:	M.M.		
SHEET # 5	S 4		
SHEET TITLE: GROUND FLOOR PLAN			

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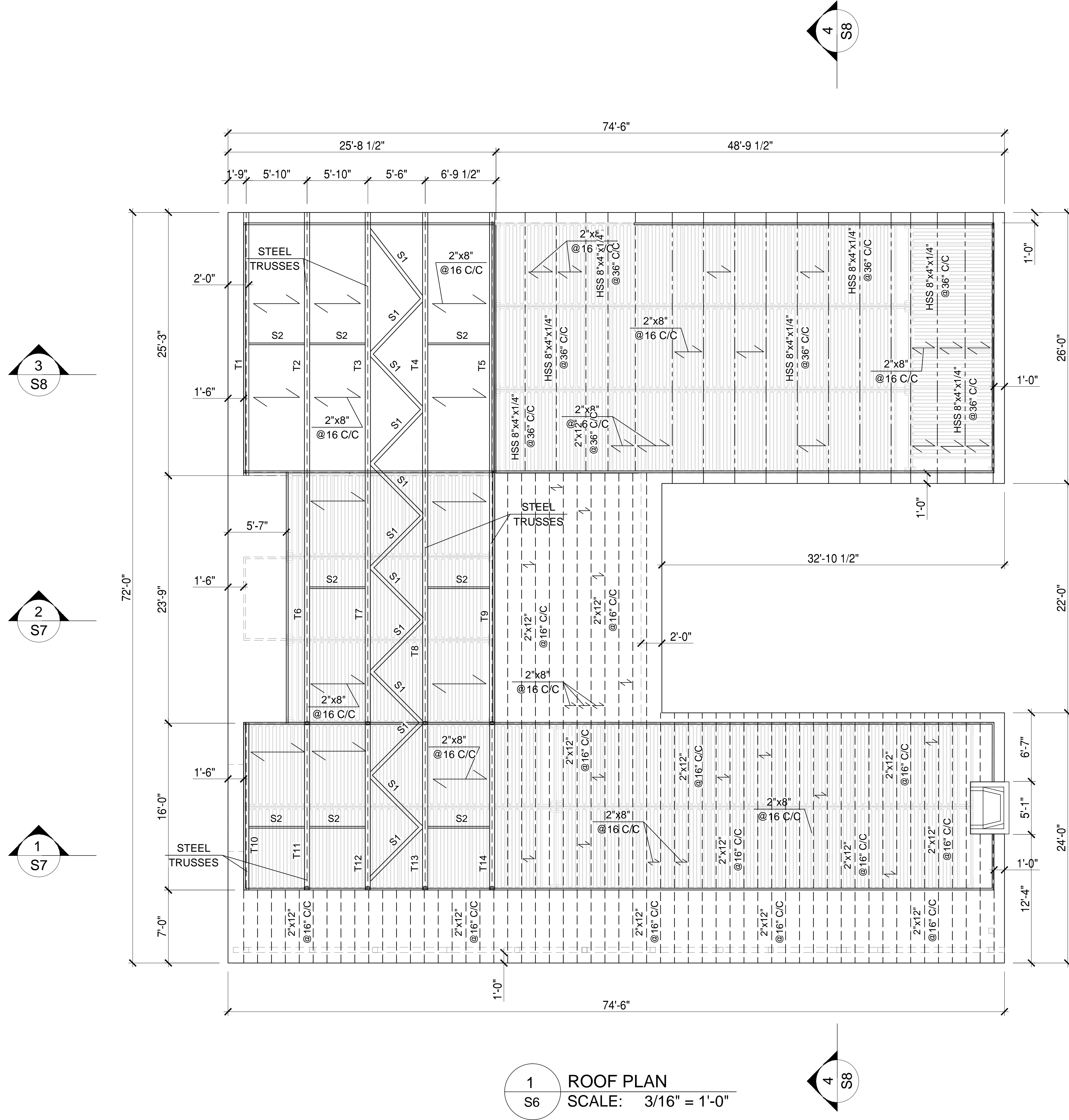
1 FIRST FLOOR PLAN
 SS SCALE: 1/4" = 1'-0"



Hanger height must be a minimum of 60% of joist depth

Web stiffeners required if sides of hanger do not laterally support at least 3/8" of TJI® joist top flange

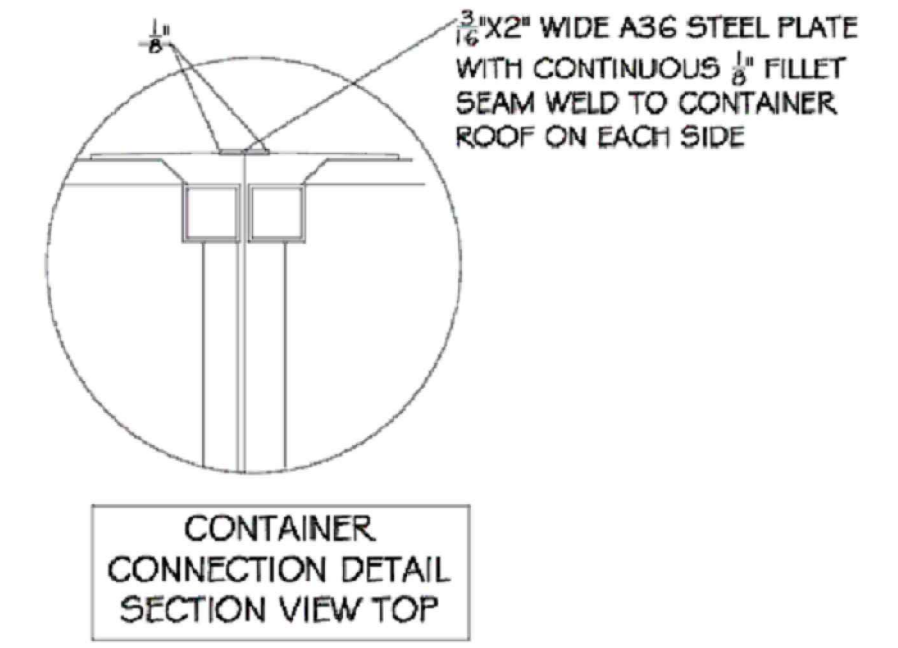
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1 ROOF PLAN
S6 SCALE: 3/16" = 1'-0"

STEEL ELEMENTS SCHEDULE

Mark	PROFILE	GRADE
S1	HSS 2"x2"x $\frac{3}{8}$ "	A53 Gr.B
S2	HSS 2"x2"x $\frac{3}{8}$ "	A53 Gr.B



CONTAINER CONNECTION DETAIL SECTION VIEW TOP

CONTAINER CONNECTION DETAIL SECTION VIEW TOP

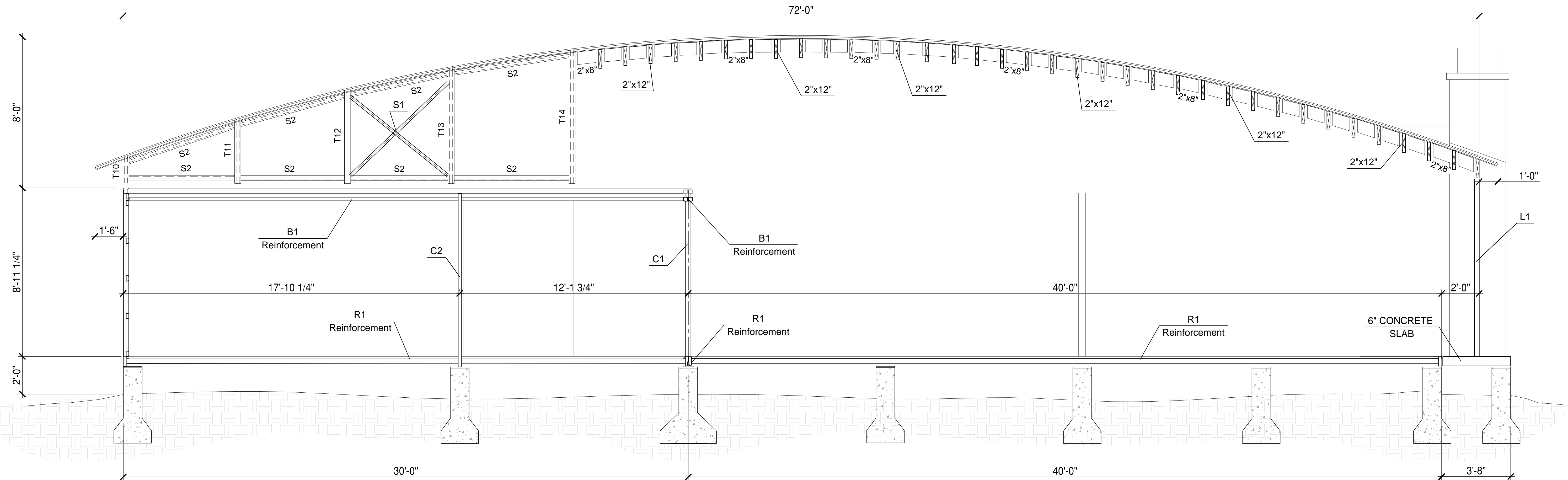
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KINGS PARK, NEW YORK 11754
TEL : 646 558 1332, FAX : 646 558 1338

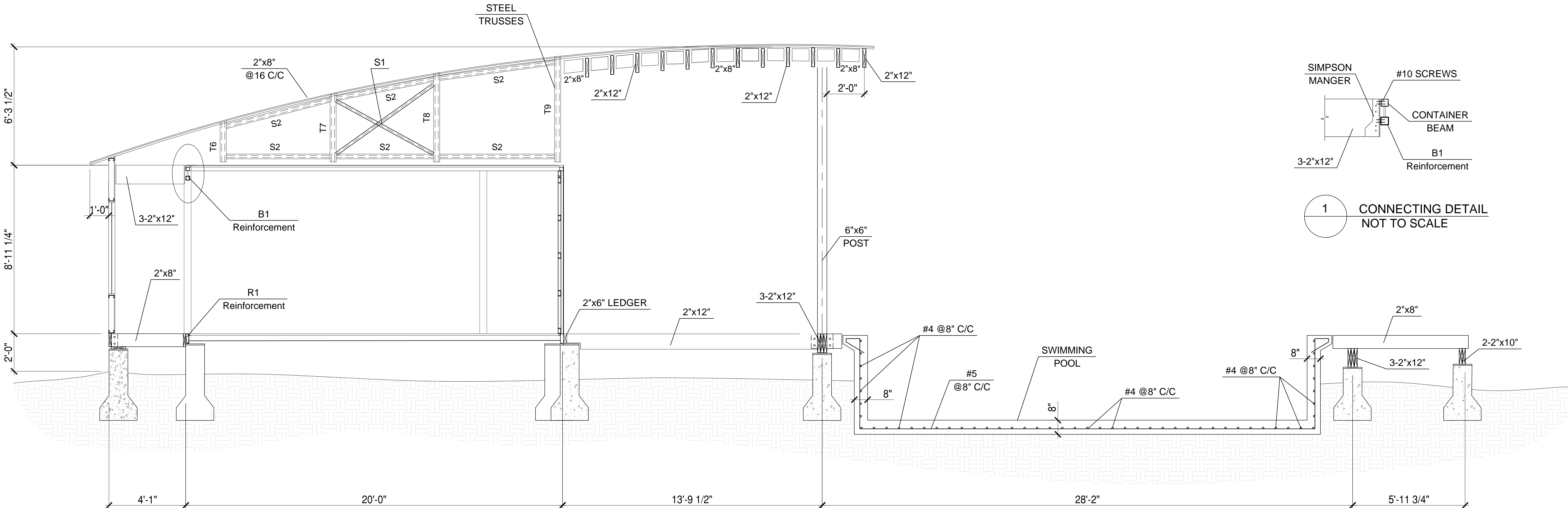
GRANT RESIDENCE
108 3RD STREET
ST. AUGUSTINE, FL 32084

JOB #	2022	DATE	9/16/22
DRAWN BY:	V.H.		
DESIGNED BY:	M.M.		
SHEET # 5	S 6		
SHEET TITLE:	ROOF PLAN		

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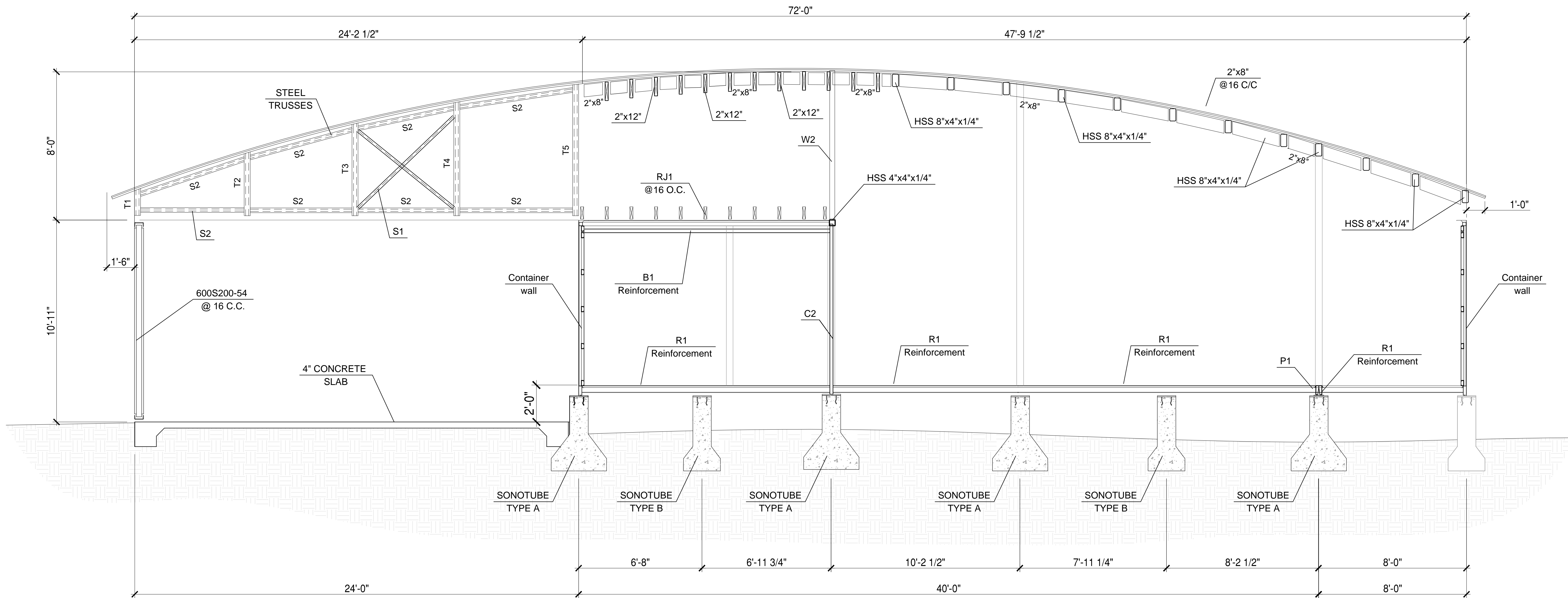


SECTION 1

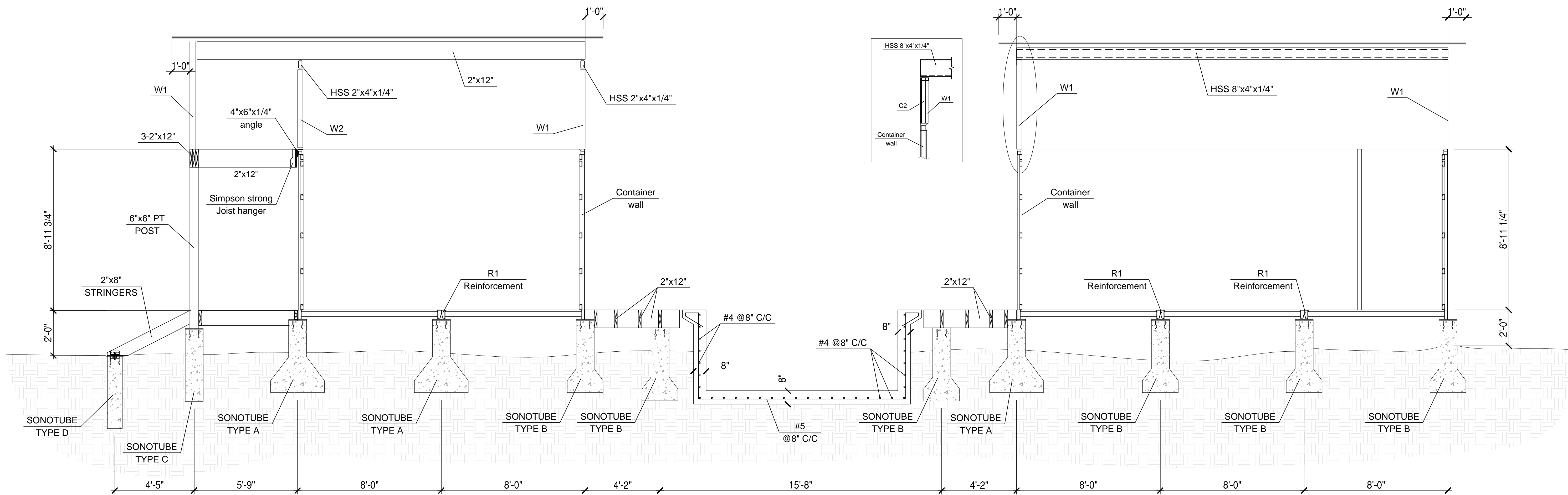


SECTION 2

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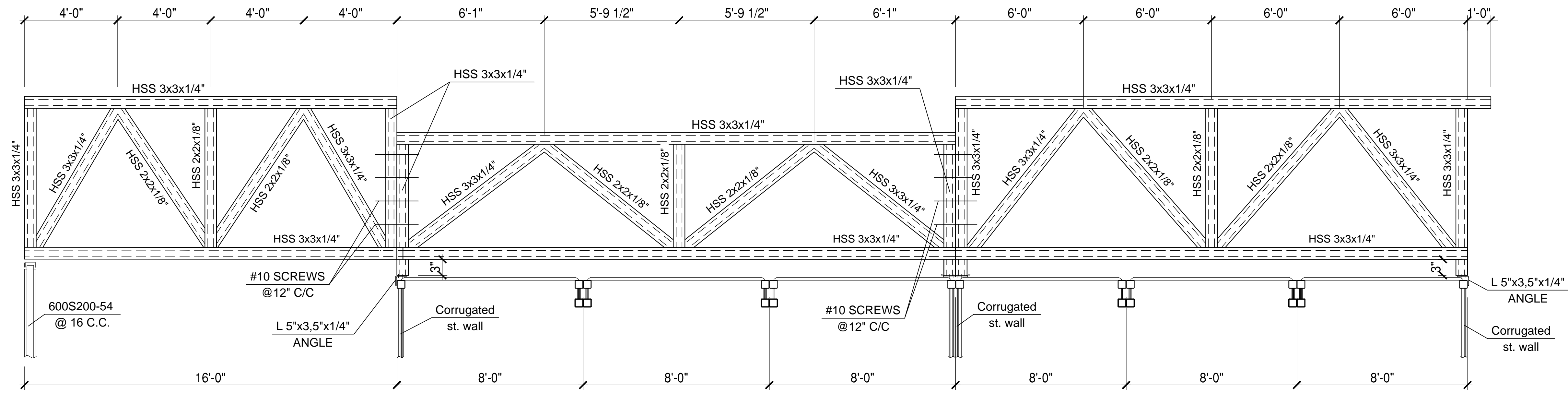
SECTION 3



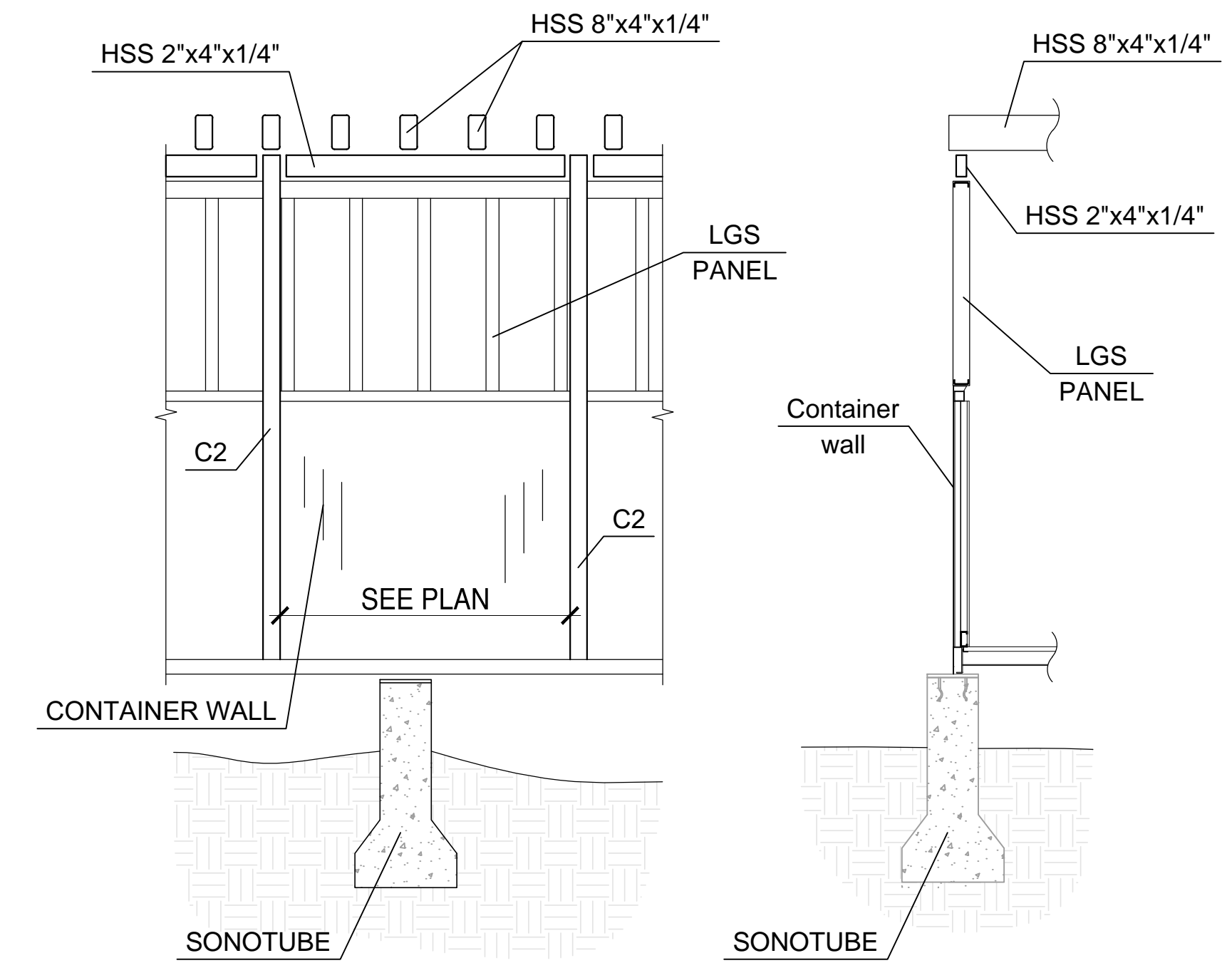
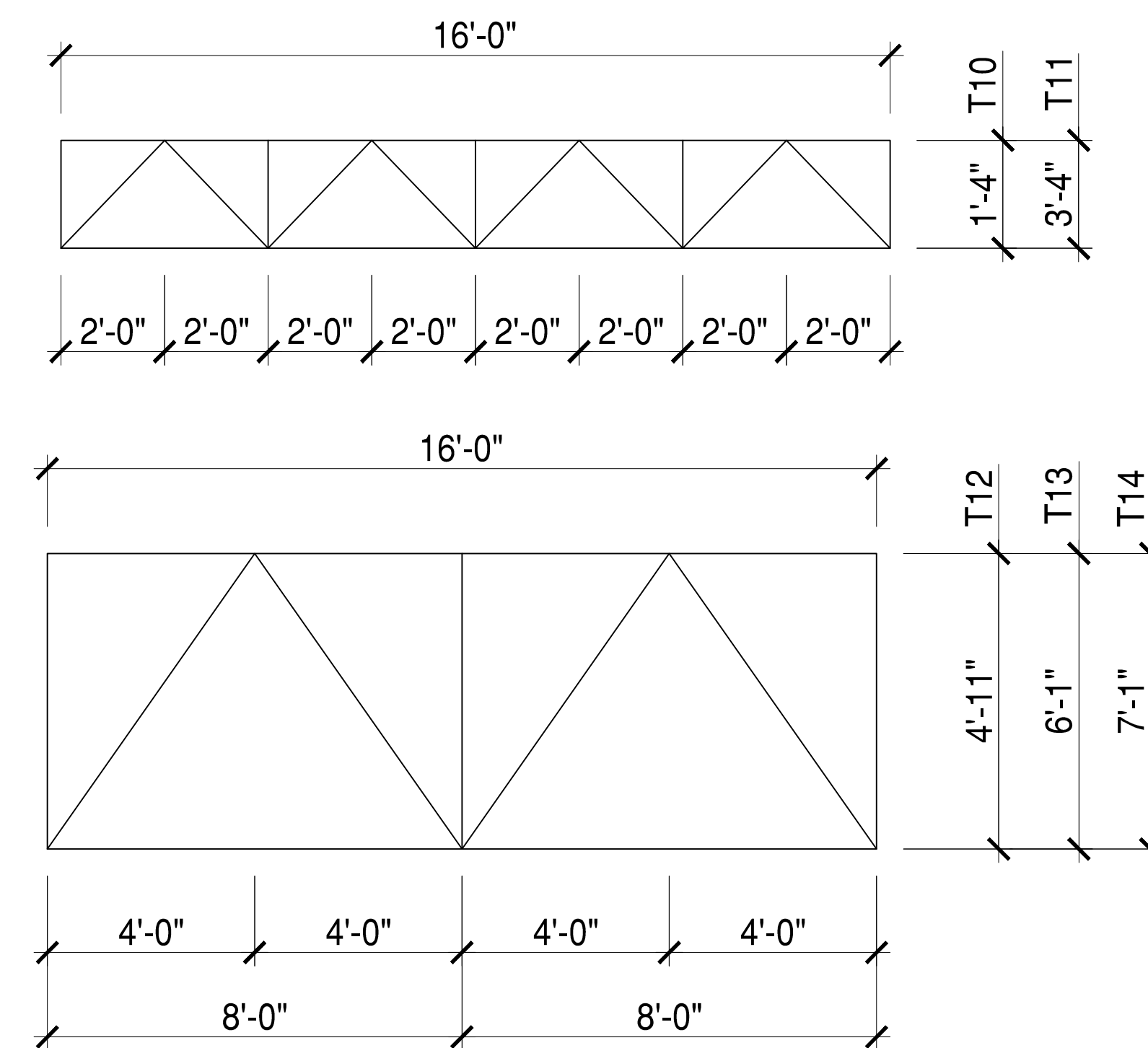
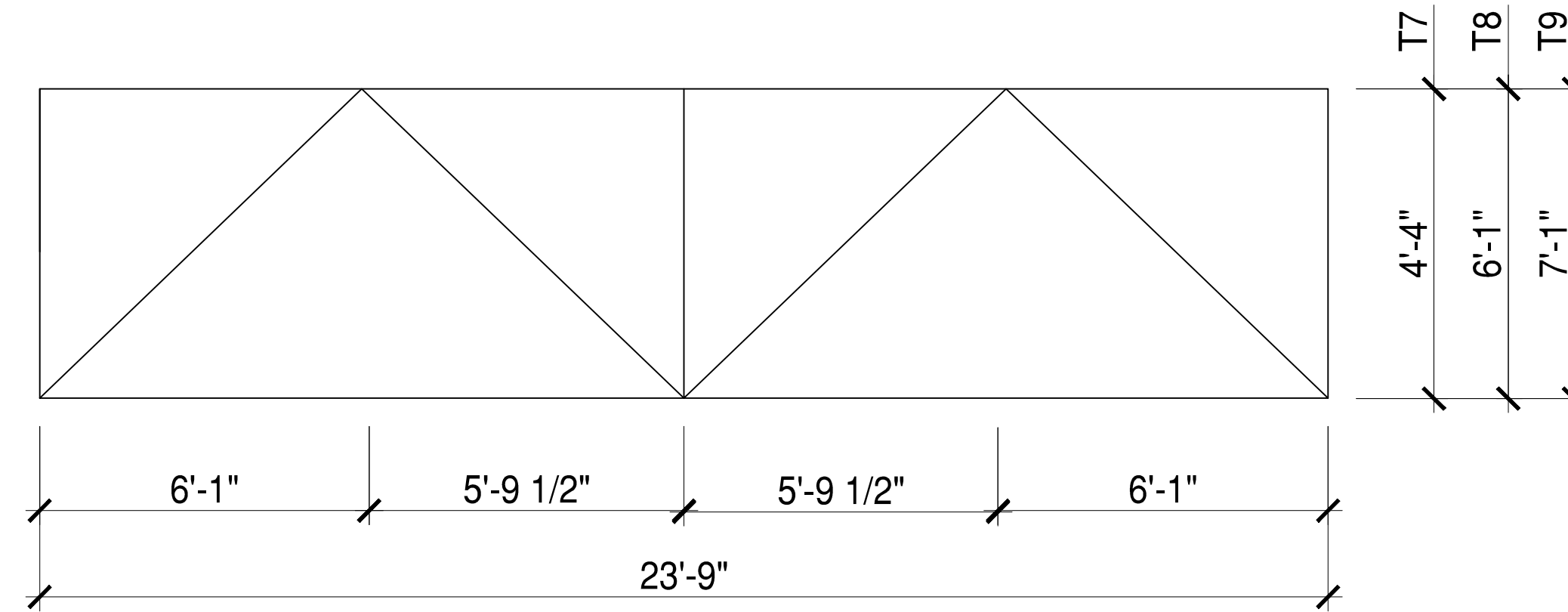
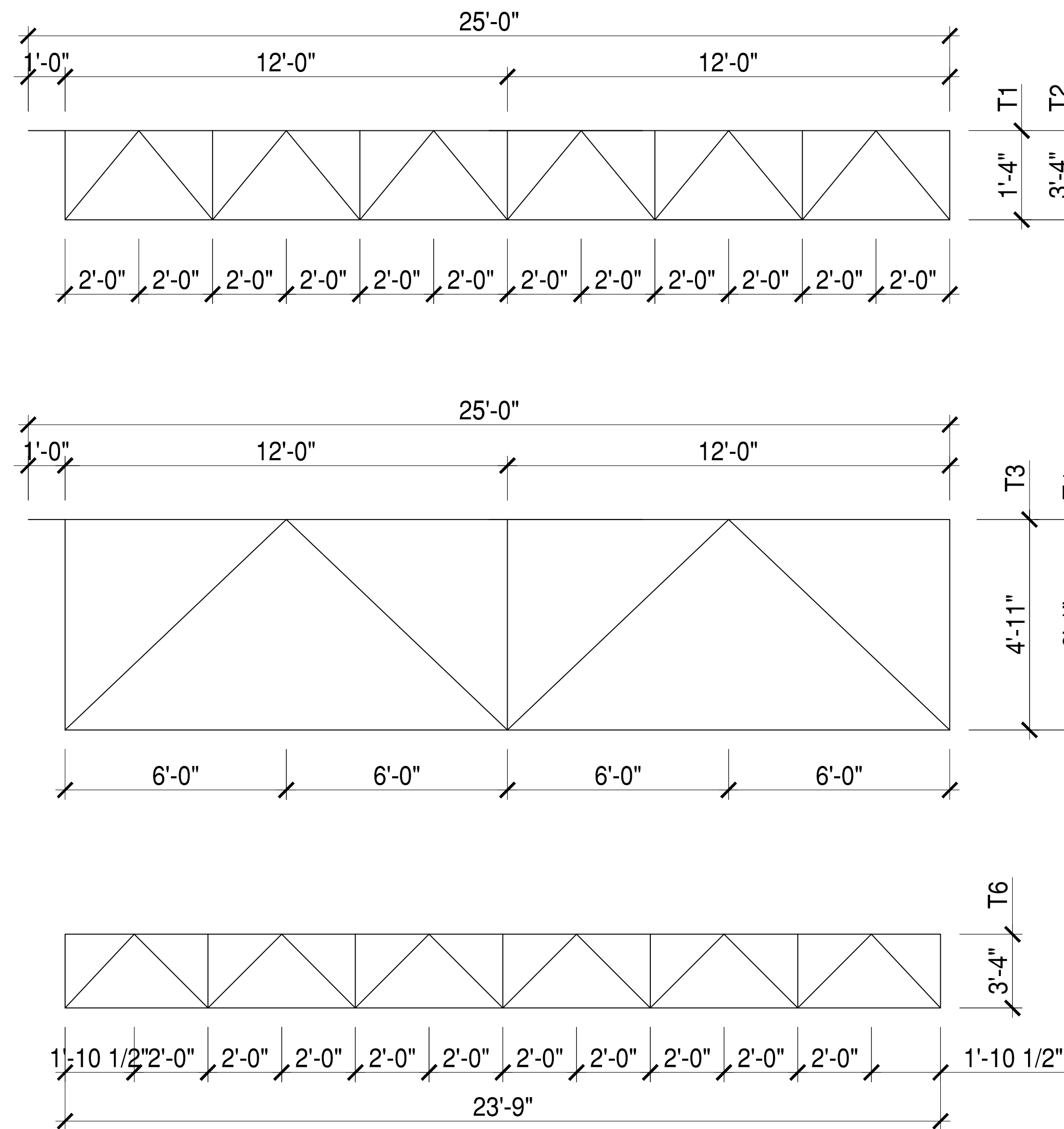
SECTION 4

JOB #	2022	DATE	9/16/22
DRAWN BY:	V.H.	DESIGNED BY:	M.M.
SHEET # 5	S 8		
SECTION 3			

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TYP. STEEL TRUSS



SECTION 5