

KIND PROJECT





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KIND PROJECT

THE KIND PROJECT INVESTORS, LP

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OPE OF WORK: TWO NEW THREE STORY APARTMENT BUILDINGS WITH CROSS- LAMINATED TIMBER FLOORS AND ROOF PANELS OVER WOOD STICK FRAMED WALLS.	3. SPECIAL INSPE
SHEATHED LIGHT FRAMED SHEAR WALLS. THE FOUNDATIONS CONSIST OF A MAT SLAB ON COMPACTED INSITU SOILS.	A. PROVIDE SI BY THE GO INSPECTIOI NOTED IN T
ESE DRAWINGS IS IN ACCORDANCE WITH THE 2019 CALIFORNIA BUILDING CODE	B. THE OWNE QUALIFIED TESTING AN
THE LATERAL FORCE RESISTING SYSTEM SHOWN ON THESE DRAWINGS IS DESIGNED TO ACHIEVE MINIMUM REQUIRED STANDARDS FOR STRUCTURAL SEISMIC RESISTANCE AND IS INTENDED TO REDUCE THE RISK OF LIFE LOSS OR INJURY. THIS WORK WILL NOT NECESSARILY PREVENT LOSS OF LIFE OR INJURY, NOR PREVENT EARTHQUAKE DAMAGE TO NEW OR REHABILITATED BUILDINGS.	C. IF INITIAL T REVEAL TH DOCUMENT BE MADE A NOTIFY THE NOTIFICATI AND SHALL
<u>GENERAL</u> MATERIALS AND WORKMANSHIP TO CONFORM TO THE BUILDING CODE DEFINED ABOVE	D. SPECIAL IN
AND THE REQUIREMENTS OF THE CONTRACT DOCUMENTS.	- OF COMPLE STRUCTUR
A. THESE NOTES APPLY TO ALL DRAWINGS AND GOVERN UNLESS OTHERWISE NOTED OR SPECIFIED. WHENEVER THERE APPEARS TO BE A CONFLICT BETWEEN THE NOTES, DRAWINGS, OR SPECIFICATIONS, CONTACT THE OWNER'S REPRESENTATIVE/ENGINEER FOR CLARIFICATION.	E. THE CONTR PRIOR TO T F. THE FOLLO
B. CONTRACTOR SHALL VERIFY ALL EXISTING CONDITIONS AND PROPOSED DIMENSIONS AT JOB SITE. COMPARE STRUCTURAL DRAWINGS WITH ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS BEFORE COMMENCING WORK. NOTIFY OWNER'S REPRESENTATIVE/ENGINEER OF ANY DISCREPANCIES AND DO NOT PROCEED WITH AFFECTED WORK UNTIL THEY ARE RESOLVED. DO NOT SCALE DRAWINGS.	THE TESTIN 1) CONCRI a. SAM 1 I 2 I 3 I
C. DRAWINGS INDICATE GENERAL AND TYPICAL DETAILS OF CONSTRUCTION. WHERE CONDITIONS ARE NOT SPECIFICALLY INDICATED BUT ARE OF SIMILAR CHARACTER TO DETAILS SHOWN, USE SIMILAR DETAILS OF CONSTRUCTION, SUBJECT TO REVIEW AND APPROVAL BY THE ENGINEER.	b. REIN 1 I 2 (3 N 6 CON
D. DETAILS NOTED AS "TYPICAL" IN THEIR TITLE OR ON SHEETS TITLED "TYPICAL DETAILS" APPLY TO SITUATIONS OCCURRING ON THE PROJECT THAT ARE THE SAME OR SIMILAR TO THOSE SPECIFICALLY REFERENCED. SUCH DETAILS ARE NOT NOTED AT EACH LOCATION THAT THEY OCCUR.	d. CAS e. CUR f. REV g. REV h. FOR
E. ALL ELEMENTS INDICATED ON THE DRAWINGS SHALL BE ASSUMED "NEW" UNLESS OTHERWISE NOTED.	1 S 2 I 2 I
F. SAFETY MEASURES: AT ALL TIMES THE CONTRACTOR SHALL BE SOLELY AND COMPLETELY RESPONSIBLE FOR THE CONDITIONS OF THE JOB SITE INCLUDING, BUT NOT LIMITED TO: a) SAFETY OF THE PERSONS AND PROPERTY.	2) NON-SH a. PLA b. CAS
b) MEANS AND METHODS OF CONSTRUCTION, c) COMPLIANCE WITH APPLICABLE CAL/OSHA REQUIREMENTS AND GUIDELINES,	3) ALL STF FOLLOV
THE CONTRACTOR SHALL BRACE OR SHORE THE CONSTRUCTION AS REQUIRED TO	a. CON PAR INCI
PROVIDE A SAFE AND TRUE STRUCTURE. WHERE BRACING OR SHORING IS INDICATED IN THE DRAWINGS, IT IS DONE SO ONLY AS A COURTESY TO THE CONTRACTOR AND SHALL NOT RELIEVE THE CONTRACTOR OF THEIR	b. CON c. PER
RESPONSIBILITY TO COORDINATE THE WORK WITH THE AFOREMENTIONED PROVISIONS. THE ARCHITECT'S OR ENGINEER'S JOB SITE REVIEW IS NOT INTENDED) 1 5 2 1
TO INCLUDE REVIEW OF THE ADEQUACY OF THE CONTRACTOR'S SAFETY MEASURES.	4) POST IN TENSIO
SUBMITTALS	FOR LO a. CON
A. SUBMIT (1) HARDCOPY OR ELECTRONIC PORTABLE DOCUMENT FORMAT (PDF) COPY OF REQUIRED SUBMITTALS TO OWNER'S REPRESENTATIVE FOR REVIEW. MULTIPLE CODIES OF THE SAME SUBMITTAL WILL NOT BE DETURNED. THE CONTRACTOR	Y 2 1
COPIES OF THE SAME SUBMITTAL WILL NOT BE RETURNED. THE CONTRACTOR SHALL ASSUME RESPONSIBILITY FOR MAKING ANY ADDITIONAL COPIES OF REVIEWED SUBMITTALS, AS MAY BE REQUIRED. THE ENGINEER SHALL HAVE 15 WORKING DAYS FROM DATE OF RECEIPT TO COMPLETE AND RETURN THE SUBMITTAL REVIEW.	5) STRUCT a. PER ANC SEIS
B. SUBSTITUTION REQUESTS SHALL DEMONSTRATE THE REQUESTED SUBSTITUTION'S	S STR
SPECIFIED ITEM. THE REQUEST SHALL ALSO INCLUDE A ROUGH COST SAVINGS ESTIMATE TO THE OWNER, REFERENCES TO DETAILS WHERE SUBSTITUTION IS PROPOSED TO BE APPLIED, AND ALL SUPPORTING DOCUMENTATION REQUIRED FO THE ITEM BY THIS SECTION OF THE NOTES.	6) ALL EXC BUILDIN AND/OR
C. SHOP DRAWINGS, MILL CERTIFICATES, AND/OR OTHER RELEVANT CERTIFICATIONS SHALL BE SUBMITTED FOR REVIEW AND APPROVAL BEFORE FABRICATION, FOR THE FOLLOWING ITEMS:	<u>4. STRUCTURAL (</u> A. STRUCTUR
NOTE: SUBMITTING COPIES OF THE STRUCTURAL DRAWINGS IS UNACCEPTABLE AND WILL BE REJECTED FOR COMPLETE REVISION.	THE SUPER ARE SEPAR
1) CROSS-LAMINATED TIMBER, GLUED LAMINATED BEAMS AND PREFABRICATED	B. THE PURPC
a. SHOP DRAWINGS INDICATING FRAMING SIZES AND SPACING OF MEMBERS, CAMBERS, CONNECTION INFORMATION, APPEARANCE	PROGRESS THE CONST
CLASSIFICATION, ETC. b. MATERIAL CERTIFICATES, INCLUDING GRADE, STRENGTH, AND STIEFNESS	REGULAR S
2) CAST-IN-PLACE CONCRETE AND SHOTCRETE	C. UNLESS OT ENGAGED
a. MIX DESIGNS FOR EACH TYPE OF CONCRETE ON THE PROJECT INCLUDING RESULTS OF SLUMP, COMPRESSION, AND SHRINKAGE	CONSTRUC
 TESTS AND OTHER PROJECT SPECIFIC CRITERIA b. MATERIAL CERTIFICATES c. PROPOSED CONSTRUCTION AND CONTROL JOINT LOCATIONS 	1) FOU 2) STR
d. CURING MATERIALS AND METHODS e. PRODUCT DATA FOR NON-SHRINK GROUT	3) LAT 4) WO
I. FORIVIVORK TYPE, FORIVIVORK, JOINT LOCATIONS, CHAIRS, FORM TIES, ETC. g. PROPOSED ROUGHENING METHODS AND TECHNIQUES TO PREPARE	ADDITIC THAT EI
EXISTING SURFACES TO RECEIVE NEW CONCRETE, IN ACCORDANCE WITH AMPLITUDE NOTED IN THE CONCRETE SECTION OF THESE NOTES.	D. THE CONTE TIME OF OE
 3) UNIT MASONRY a. MASONRY UNIT MANUFACTURERS MATERIAL CERTIFICATION 	E. AN OWNER
 b. GROUT MIX DESIGN INCLUDING SLUMP AND COMPRESSION TESTS c. PROPOSED CONSTRUCTION JOINT LOCATIONS 	AUTHORITY NORMAL DI
 4) MECHANICAL ANCHORS AND EPOXY ANCHORS a. PRODUCT DATA FOR EACH TYPE OF SYSTEM INCLUDING ANCHOR 	
TESTING IN ACCORDANCE WITH ACI 355.2 FOR MECHANICAL ANCHORS AND ACI 355.4 FOR EPOXY ANCHORS.	
D. CERTIFICATION OF ANCHOR INSTALLERS PER ACI/CRSI WHERE ANCHORS ARE INSTALLED IN HORIZONTAL OR VERTICAL CONDITIONS WITH SUSTAINED TENSION.	
5) UNDERSLAB VAPOR-BARRIER	
6) DEFERRED DESIGN SUBMITTALS SHALL BE SUBMITTED TO THE ENGINEER AND ARCHITECT FOR REVIEW AND APPROVAL PRIOR TO SUBMISSION TO THE	
AUTHORITY HAVING JURISDICTION FOR PLAN CHECK AND BUILDING PERMIT: a. EXTERIOR CLADDING b. TIE DOWN SYSTEM OF WOOD WALLS. c. ROOF TIE-OFFS, WINDOW WASHING EQUIPMENT AND MECHANICAL UNIT	

GENERAL STRUCTURAL NOTES

SUPPORT

f. SUSPENDED CEILINGS

d. SKYLIGHT, ROOF, HATCHES & METAL LADDERS

e. STEEL STAIRS, INCLIDING SUPPORT CAGE, HANDRAILS AND GUARDRAILS

ECTION REQUIREMENTS AND TESTING

PECIAL INSPECTIONS AND TESTING FOR ALL ITEMS AS REQUIRED VERNING JURISDICTION. JURISDICTION SPECIFIC SPECIAL N FORM SHALL SUPPLEMENT SPECIAL INSPECTION REQUIREMENTS HIS SECTION.

R SHALL BE RESPONSIBLE FOR RETAINING AN INDEPENDENT, INSPECTOR AND/OR TESTING LAB TO PERFORM ALL REQUIRED ND SPECIAL INSPECTIONS.

ESTS OR INSPECTIONS MADE BY THE OWNER'S TESTING AGENCY AT ANY PORTION OF THE WORK DOES NOT COMPLY WITH THE CONTRACT S, ADDITIONAL TESTS, INSPECTIONS, AND NECESSARY REPAIRS WILL T THE CONTRACTOR'S EXPENSE. CONTRACTOR SHALL IMMEDIATELY E ENGINEER AND OWNER OF NON-CONFORMING WORK. THIS ON SHALL SPECIFICALLY ADDRESS THE NON-CONFORMING WORK BE SEPARATE FROM THE SPECIAL INSPECTION REPORTS.

SPECTION REPORTS SHALL BE SENT TO THE ENGINEER AT THE TIME ETION FOR REVIEW OF CONFORMANCE WITH THE REQUIREMENTS OF THE AL DRAWINGS.

RACTOR SHALL NOTIFY THE TESTING LAB A MINIMUM OF 48 HOURS TIME OF INSPECTION.

WING SPECIFIC ITEMS SHALL BE INSPECTED AND/OR TESTED BY NG LAB:

ETE:

IPLE AND TEST CONCRETE AS FOLLOWS:

- FABRICATE SPECIMENS FOR STRENGTH TESTS PER ACI 318. PERFORM SLUMP AND AIR CONTENT TESTS.
- DETERMINE TEMPERATURE OF THE CONCRETE.
- NFORCING STEEL PLACEMENT (CONTINUOUS INSPECTION FOR SPECIAL MOMENT FRAMES) OBTAIN AND REVIEW MILL TEST REPORTS.
- WELDING. ICRETE PLACEMENT (CONTINUOUS INSPECTION).
- T-IN-PLACE ANCHOR BOLTS.
- RING TEMPERATURE AND TECHNIQUES AND DURATION. IEW MIX DESIGN FOR EACH CLASS OF CONCRETE.
- IEW THE TICKET OF EACH BATCH OF CONCRETE DELIVERED. RMWORK (INCLUDING FORM REMOVAL AND RESHORES) SHAPE
- LOCATION
- DIMENSIONS

IRINK GROUT CEMENT

- ST AND TEST SPECIMENS FOR COMPRESSION STRENGTH
- RUCTURAL WELDING INCLUDING, BUT NOT LIMITED TO THE
- VING: ITINUOUS INSPECTION FOR ALL BUTT WELDS, COMPLETE AND TIAL PENETRATION WELDS, GROOVE WELDS AND PLUG WELDS UDING WELDING OF REINFORCEMENT.
- ITINUOUS INSPECTION OF ALL FILLET WELDS EXCEEDING 5/16". RIODIC VISUAL INSPECTION OF THE FOLLOWING ITEMS:
- SINGLE-PASS FILLET WELDS NOT EXCEEDING 5/16". WELDING OF STAIRS AND RAILING SYSTEMS.

ISTALLED ANCHORS. WHERE ANCHORS ARE LOADED IN SUSTAINED N, INSPECTION SHALL BE CONTINUOUS. REFER TO THE DRAWINGS CATIONS.

- ICRETE
- EPOXY REBAR AND THREADED RODS MECHANICAL ANCHORS

FURAL WOOD

NODIC SPECIAL INSPECTION FOR NAILING, BOLTING. CHORING AND OTHER FASTENING OF COMPONENTS WITHIN THE SMIC FORCE RESISTING SYSTEM, INCLUDING WOOD SHEAR LS. WOOD DIAPHRAGMS DRAG STRUTS. CLT PLYWOOD SPLINES. STEEL APS, BRACES, SHEAR PANELS AND HOLD-DOWNS.

CAVATIONS AND EARTH FORMS SHALL BE INSPECTED BY THE LOCAL IG INSPECTOR AND INSPECTED BY THE GEOTECHNICAL ENGINEER R ENGINEER PRIOR TO PLACING CONCRETE.

<u>OBSERVATIONS</u>

AL OBSERVATIONS WILL BE UNDERTAKEN BY PERSONNEL UNDER **RVISION OF THE ENGINEER OF RECORD. STRUCTURAL OBSERVATIONS** RATE FROM THE SPECIAL INSPECTION REQUIREMENTS OUTLINED

DSE OF STRUCTURAL OBSERVATIONS IS TO REVIEW THE OVERALL OF CONSTRUCTION AND ASCERTAIN ITS GENERAL COMPLIANCE WITH FRUCTION DOCUMENTS, THESE GENERAL NOTES, AND OTHER TIONS, WHERE APPLICABLE. OBSERVATIONS WILL BE NOTED IN SITE REPORTS ISSUED TO THE OWNER'S REPRESENTATIVE/OWNER.

HERWISE AGREED UPON, THE ENGINEER OF RECORD SHALL BE TO PROVIDE, AT MINIMUM, A LEVEL OF CONSTRUCTION INVOLVEMENT OBSERVE THE FOLLOWING AT SIGNIFICANT MILESTONES DURING THE TION PROCESS:

INDATION REINFORCEMENT AND CONSTRUCTION UCTURAL STEEL FRAMING ERAL FORCE RESISTING ELEMENTS

OD FRAMING ONAL ENGINEER INVOLVEMENT MAY BE DESIRED. ANY AGREEMENT TO

FFECT SHALL BE MADE PRIOR TO THE START OF CONSTRUCTION. RACTOR SHALL NOTIFY THE ENGINEER A MINIMUM OF 3 DAYS PRIOR TO

3SERVATION AND PROVIDE ACCESS FOR THE OBSERVATIONS. 'S REPRESENTATIVE MAY BE DESIGNATED, BY THE OWNER'S SPECIFIC

ATION PRIOR TO THE START OF CONSTRUCTION. WHO WILL HAVE THE TO REQUEST ADDITIONAL ENGINEER INVOLVEMENT OUTSIDE OF THE JTIES ASSOCIATED WITH STRUCTURAL OBSERVATION.

5. DESIGN BASIS

- A. CONSTRUCT IN CONFORMANCE WITH THE BUILDING CODE NOTED ABOVE.
- B. DESIGN LIVE LOADS (PSF):
- ROOF FLOOR 40 CORRIDOR 100
- DECKS C. DESIGN DEAD LOADS 1) SUPERIMPOSED DEAD LOADS NOTED ON PLANS
- D. EARTHQUAKE DESIGN DATA
- 1) SEISMIC IMPORTANCE FACTOR, l: 1.0 RISK CATEGORY:
- 3) USGS MCEr SPECTRAL RESPONSE ACCELERATIONS: i. Ss = 0.58 gii. S1 = 0.256 g
- 4) SITE CLASS: D-STIFF SOIL 5) ASCE 7 DESIGN SPECTRAL RESPONSE ACCELERATIONS: i. SDS = 0.517 g
- ii. SD1 = 0.357 g6) SEISMIC DESIGN CATEGORY:
- BASIC SEISMIC-FORCE RESISTING SYSTEM:
- 8) RESPONSE MODIFICATION FACTOR, R:
- 9) SEISMIC RESPONSE COEFFICIENT, Cs (AT STRENGTH LEVEL):
- 10) DESIGN BASE SHEAR: 11) ANALYSIS PROCEDURE USED:
- 12) DESIGN STORY DRIFT:
- E. WIND:
- 1) RISK CATEGORY:
- 2) BASIC WIND SPEED:
- 3) WIND DIRECTIONALITY FACTOR, Kd: EXPOSURE CATEGORY TYPE:
- 5) TOPOGRAPHIC FACTOR, Kzt:
- F. FOUNDATIONS: MODIFY AS REQ'D
- 1) MAT SLAB:

FOUNDATION, FILL, AND SITE WORK

- FOUNDATION DESIGN IS BASED ON A GEOTECHNICAL REPORT PREPARED BY: GEOCON DATED: AUGUST 2020
- A. EXCEPT WHERE OTHERWISE SHOWN, EXCAVATIONS SHALL BE MADE AS NEAR AS POSSIBLE TO THE NEAT LINES REQUIRED BY THE SIZE AND SHAPE OF THE STRUCTURE. ALL FOUNDATIONS SHALL BE POURED WITHOUT THE USE OF SIDE FORMS WHEREVER POSSIBLE. IF THE TRENCHES CANNOT STAND. FULLY FORM SIDES TO DIMENSIONS SHOWN.
- B. DO NOT ALLOW WATER TO STAND IN TRENCHES. IF BOTTOMS OF TRENCHES BECOME SOFTENED DUE TO RAIN OR SLURRY OR OTHER WATER BEFORE CONCRETE IS CAST, EXCAVATE SOFTENED MATERIAL AND REPLACE WITH PROPERLY COMPACTED BACKFILL OR CONCRETE AT NO COST TO OWNER.
- C. WHERE SITEWORK IS REQUIRED. COMPLY WITH THE FOLLOWING:
- 1) STRIP THE AREA TO BE BUILT OVER OF ALL ORGANIC MATERIAL AND TOP
- 2) SCARIFY THE TOP 6 INCHES OF STRIPPED SURFACE; BRING TO CORRECT MOISTURE CONTENT; THEN RE-COMPACT TO AT LEAST 95% UNDER FOOTINGS AND 90% ELSEWHERE
- 3) FILL MATERIAL TO BE PLACED IN 6 INCH LAYERS AND COMPACTED. 4) FILL MATERIAL SHALL BE FREE OF PLASTIC CLAYS, VEGETATION, AND OTHER DELETERIOUS MATERIAL; IT SHALL BE OF SUCH QUALITY THAT IT WILL COMPACT THOROUGHLY WHEN WATERED AND ROLLED. THE FILL SHALL NOT CONTAIN ROCKS OR LUMPS OVER 2 INCHES IN GREATEST DIMENSION.
- D. PLACE BACKFILL BEHIND RETAINING WALLS AFTER CONCRETE HAS ATTAINED FULL DESIGN STRENGTH. BRACE BUILDING AND PIT WALLS BELOW GRADE FROM LATERAL LOADS UNTIL ATTACHED FLOORS AND SLABS ON GRADE HAVE ATTAINED FULL DESIGN STRENGTH.
- . FOR SHALLOW FOUNDATIONS, THE TOP SURFACE OF FOOTINGS SHALL BE LEVEL. THE BOTTOM SURFACE OF FOOTINGS SHALL BE PERMITTED TO HAVE A SLOPE NOT EXCEEDING ONE UNIT VERTICAL IN 10 UNITS HORIZONTAL (10-PERCENT SLOPE). FOOTINGS SHALL BE STEPPED WHERE IT IS NECESSARY TO CHANGE THE ELEVATION OF THE TOP SURFACE OF THE FOOTING OR WHERE THE SURFACE OF THE GROUND SLOPES MORE THAN ONE UNIT VERTICAL IN 10 UNITS HORIZONTAL.
- 8. CONCRETE
- A. EXCEPT WHERE NOTED OTHERWISE ALL WORK SHALL BE PERFORMED IN ACCORDANCE WITH THE LATEST EDITION OF ACI 301 - SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS. UNLESS OTHERWISE NOTED, COMPLY WITH CONSTRUCTION TOLERANCES AS SPECIFIED IN ACI 117 "SPECIFICATION FOR TOLERANCES FOR CONCRETE CONSTRUCTION AND MATERIALS".
- B. REINFORCE ALL CONCRETE. INSTALL ALL INSERTS, BOLTS, ANCHORS, AND REINFORCING AND SECURELY TIE PRIOR TO PLACING CONCRETE.
- C. PORTLAND CEMENT SHALL CONFORM TO ASTM C-150 TYPE I OR II. NOTE: UPDATE CEMENT TYPE BASED ON GEOTECHNICAL REQUIREMENTS/SOIL CORROSIVITY.
- D. CONCRETE SHALL BE HARDROCK CONCRETE AND CONFORM TO ALL REQUIREMENTS OF ASTM C-33. UNLESS OTHERWISE NOTED. WHERE LIGHTWEIGHT CONCRETE IS SPECIFIED, IT SHALL CONFORM TO ASTM C-330. FLY ASH SHALL COMPLY WITH ASTM C618; SLAG SHALL COMPLY WITH ASTM C989. PROPORTION CONCRETE IN ACCORDANCE WITH ACI 211.1, INCLUDING ANY REQUIRED ADMIXTURES. CONCRETE SHALL SATISFY THE FOLLOWING PROPERTIES:

ADMIXTURES WITH CHLORIDE IONS: MIN. FLY ASH OR SLAG REPLACEMENT: MAX. SHRINKAGE AT 28 DAYS:

LOCATION	MIN. STRENGTH	MAX. AGGREGATE	MAX. SLUMP
	@ 28 DAYS PSI	SIZE - INCHES	INCHES
MAT FOUNDATION	4000	1-1/2"	4

E. THE ACTUAL SLUMP AND TOLERANCE SHALL BE ESTABLISHED BY THE CONTRACTOR AND CONCRETE SUPPLIER, AS REQUIRED TO SATISFY THE CONTRACTOR'S MEANS AND-METHODS FOR PLACEMENT, FIELD AND INSTALLATION CONDITIONS (INCLUDING REINFORCING CONGESTION), FINISH REQUIREMENTS, AND AS REQUIRED TO SATISFY THE PERFORMANCE CRITERIA SPECIFIED ABOVE.

- 1.0
- 110 MPH 0.85

- 6) ENCLOSURE CLASSIFICATION:



- ENCLOSED BUILDING
- 1,500 PSF (1/3 INCREASE FOR SEISMIC LOADING)

- NOT PERMITTED 20% 0.040% PER ASTM C157
- (SEAONC METHOD)

- F. WHEN PLACING NEW CONCRETE AGAINST EXISTING CONCRETE AND/OR CONCRETE MASONRY, ROUGHEN EXISTING MATERIAL TO 1/4" AMPLITUDE. REMOVE ALL LOOSE CEMENTITIOUS MATERIALS AND AGGREGATES. PRESSURE WASH SURFACE AND REMOVE STANDING WATER IMMEDIATELY PRIOR TO PLACING NEW CONCRETE.
- G. CONTRACTOR SHALL CONSTRUCT CONCRETE FLOORS AND SLABS PER RECOMMENDATIONS OF ACI 302.1R. CONTRACTOR SHALL SUBMIT LOCATIONS OF PROPOSED CONSTRUCTION JOINTS FOR ENGINEERS REVIEW AND APPROVAL.

9. FORMWORK

- A. DESIGN AND CONSTRUCT FORMWORK IN ACCORDANCE WITH ACI 347 **RECOMMENDED PRACTICE FOR CONCRETE FORMWORK**" AND ACI 301 "SPECIFICATIONS FOR STRUCTURAL CONCRETE", UNLESS OTHERWISE NOTED.
- B. AS REQUIRED, PROVIDE POUR POCKETS IN FORMS AND UNDER EXISTING MEMBERS TO PREVENT AIR POCKETS OR "HONEYCOMBS". CONCRETE CAST WITH AIR POCKETS OR HONEYCOMBS IS NOT ACCEPTABLE.
- C. PROVIDE ³/₄ INCH BY ³/₄ INCH CHAMFER STRIPS ON ALL EXTERNAL CORNERS OF BEAMS, COLUMNS, AND WALLS, UNLESS OTHERWISE NOTED.
- D. REMOVE FORMS AND SHORES IN ACCORDANCE WITH THE FOLLOWING:
- FOOTINGS, PILE CAPS, AND GRADE BEAMS REMOVE FORMS AND SHORES NO SOONER THAN 48 HOURS.
- E. CONCRETE SHALL BE CONTINUOUSLY CURED FOR 10 DAYS AFTER PLACING IN ANY APPROVED MANNER IN ACCORDANCE WITH ACI 301, INCLUDING CURING COMPOUND, CURING PAPER, WATER SPRAY, FLOODING WITH WATER (FOR SLABS), ETC. PROVIDE CURING WHERE FORMS ARE REMOVED IN LESS THAN 7 DAYS.

10. REINFORCING STEEL

- A. ALL REINFORCING STEEL BARS, UNLESS OTHERWISE NOTED, SHALL CONFORM WITH THE LATEST STANDARD SPECIFICATIONS FOR DEFORMED BILLET STEEL FOR CONCRETE REINFORCEMENT, ASTM DESIGNATION A615 AND SHALL BE MINIMUM GRADE 60. HEADED SHEAR STUD REINFORCING SHALL COMPLY WITH ASTM A1044
- B. SUITABLE DEVICES (DOBIES, CHAIRS, ETC.) OF SOME STANDARD MANUFACTURE SHALL BE USED TO HOLD REINFORCEMENTS IN ITS TRUE HORIZONTAL AND VERTICAL POSITIONS. THESE DEVICES SHALL BE SUFFICIENTLY RIGID AND NUMEROUS TO PREVENT DISPLACEMENT OF THE REINFORCING DURING PLACING OF CONCRETE. ALL SUCH DEVICES HAVE PRIOR APPROVAL FROM THE ARCHITECT AND ENGINEER.
- C. LAP SPLICE ALL BARS IN CONCRETE PER STANDARD DETAILS SCHEDULE, USING LAP TYPE "TOP" UNLESS OTHERWISE NOTED. WHEN LAPPING BARS OF DIFFERENT SIZES, USE THE LAP LENGTH OF THE LARGER BAR.
- D. UNLESS OTHERWISE DEMONSTRATED BY SUCCESSFUL PLACEMENT OF A REPRESENTATIVE TEST PANEL, LAP SPLICES FOR SHOTCRETE WALLS SHALL BE PER NON-CONTACT SPLICE METHOD. THE LAPPED BARS SHALL BE SPACED A MINIMUM OF 2 INCHES BETWEEN THEM AND THE LAP LENGTH SHALL BE PER THE SCHEDULE USING LAP CLASS B, "TOP".
- E. DETAIL ACCORDING TO THE LATEST ACI STANDARD 315, MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES. PLACE REINFORCEMENT PER ACI 301, "SPECIFICATION FOR STRUCTURAL CONCRETE", UNLESS OTHERWISE NOTED.
- F. REBAR PARTIALLY EMBEDDED IN CONCRETE SHALL NOT BE FIELD BENT.
- G. REBAR SHALL ONLY BE BENT ONCE. REBAR SHALL NOT BE BENT AND STRAIGHTENED FOR CONSTRUCTION UNLESS EXPLICITLY NOTED ON THE CONSTRUCTION DOCUMENTS.
- H. MAINTAIN COVERAGE TO FACE OF BARS, INCLUDING SLEEVES AND PENETRATIONS. AS FOLLOWS, UNLESS OTHERWISE NOTED:
- 1) CAST-IN-PLACE CONCRETE a. 3 INCHES WHERE CONCRETE IS DEPOSITED AGAINST EARTH EXCEPT
- SLAB-ON-GRADE. b. 2 INCHES FOR FORMED CONCRETE WHICH IS EXPOSED TO EARTH OR WEATHER FOR #6 BAR THROUGH #18 BAR. REDUCED TO 1-1/2 FOR #5 BAR, AND SMALLER.
- c. 3/4 INCHES FOR INTERIOR SLABS d. 1-1/2 INCHES FOR SLAB-ON-GRADE.
- **11. FRAMING LUMBER**
- A. ALL FRAMING LUMBER SHALL BE GRADED PER WCLIB GRADING RULES NO. 17.
- B. ALL FRAMING LUMBER SHALL HAVE A MAXIMUM MOISTURE CONTENT OF 19% AT
- TIME OF INSTALLATION. C. ALL POSTS AND BEAMS SHALL BE DOUGLAS FIR, #1
- D. ALL FLOOR AND ROOF JOISTS SHALL BE DOUGLAS FIR, #1
- E. ALL STUDS, PLATES, ETC., SHALL BE DOUGLAS FIR, #2.
- F. ENGINEERED WOOD PRODUCTS MAY BE USED AS SUBSTITUTES FOR SAWN LUMBER UPON REQUEST BY THE CONTRACTOR AND APPROVAL FROM THE ARCHITECT AND ENGINEER OF RECORD. CONTRACTOR SHALL SUBMIT MANUFACTURER'S TESTING REPORTS FOR APPROVAL

12. ENGINEERED WOOD PRODUCTS (EWP)

- A. ALL ENGINEERED WOOD PRODUCTS (EWP) SUPPLIED ON THIS PROJECT SHALL BE SUPPLIED BY ONE MANUFACTURER.
- B. ALL MICROLLAM LVL FRAMING MEMBERS SHALL BE FABRICATED BY TRUS JOIST WITH THE FOLLOWING ALLOWABLE STRESSES: Fb = 2600 PSI. Fv = 285 PSI. E = 2,000,000 PSI. MOISTURE CONTENT AT THE TIME OF FABRICATION SHALL NOT EXCEED 9%.
- C. ALL PARALLAM PSL FRAMING MEMBERS SHALL BE FABRICATED BY TRUS JOIST WITH THE FOLLOWING ALLOWABLE STRESSES: Fb = 2900 PSI. Fv = 290 PSI. E = 2,200,000 PSI. MOISTURE CONTENT AT THE TIME OF FABRICATION SHALL NOT EXCEED 9%.
- D. ALL TJI PREFABRICATED WOOD I-JOISTS SHALL BE FABRICATED BY TRUS JOIST.





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KIND PROJECT

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2 PLAN CHECK COMMENTS

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DATE	4/20/2021
SCALE	N.T.S.
FILENAME	
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13. GLUED-LAMINATED MEMBERS:

GLUED-LAMINATED (GLULAM) MEMBERS SHALL BE FABRICATED IN CONFORMANCE WITH ANSI STANDARD A190.1. AMERICAN NATIONAL STANDARD FOR STRUCTURAL GLUED LAMINATED TIMBER OR OTHER CODE-APPROVED DESIGN, MANUFACTURING AND/OR QUALITY ASSURANCE PROCEDURES. EACH MEMBER SHALL BEAR AN AITC OR APA-EWS IDENTIFICATION MARK OR BE ACCOMPANIED BY A CERTIFICATE OF CONFORMANCE. ONE COAT OF END SEALER SHALL BE APPLIED IMMEDIATELY AFTER TRIMMING IN EITHER THE

SHOP OR THE FIELD. GLULAM MEMBERS SHALL BE FRAMING (HIDDEN) OR ARCHITECTURAL (EXPOSED) APPEARANCE CLASSIFICATION AND OF THE STRENGTH INDICATED BELOW:

GLUED-LAMINATED MEMBERS					
COMBINATION SYMBOL (SPECIES)	USE	FLEXURAL STRESS, Fb (PSI)	MODULUS OF ELASTICITY (PSI)	HORIZONTAL SHEAR STRESS Fv (PSI)	
24F-V4 (DF/DF)	SIMPLE SPAN	+2,400/-1,850	1,800,000	265	
24F-V8 (DF/DF)	CONTINUOUS OR CANTILEVER	+/- 2,400	1,800,000	265	

ADHESIVE SHALL BE WET-USE EXTERIOR, WATERPROOF GLUE. FIELD NOTCHING AND BORING OF GLULAM MEMBERS NOT ALLOWED UNLESS APPROVED BY SER. GLULAM MEMBERS SHALL BE SUPPLIED TO THE PROJECT WITH BETWEEN 3,500 AND 5,000 FOOT STANDARD MILL CAMBER WITH TOLERANCES AS ALLOWED BY ANSI A190. THE DRAWINGS WILL INDICATE WHETHER ADDITIONAL CAMBER IS REQUIRED.

14. CROSS LAMINATED TIMBER PANELS:

3 PLY (105V)

5 PLY (175V)

CROSS LAMINATED TIMBER (CLT) MEMBERS SHALL BE MANUFACTURED IN COFORMANCE WITH ANSI/APA PRG 320-2018 STANDARD FOR PERFORMACE-RATED CROSS-LAMINATED TIMBER, DEMONSTRATION OF EQUIVALENCE SHALL BE RESPONSIBILITY OF THE MANUFACTURER. PANELS SHALL BE INDUSTRIAL (HIDDEN) OR ARCHITECTURAL (EXPOSED) WITH LAYUPS AS NOTED ON THE STRUCTURAL PLANS OF THE STRENGTHS

PANEL	_ TYPE	MII	NIMUM MAJ	OR STRENC	GTH	MI	NIMUM MIN DIRE	OR STRENC	GTH
	t (in)	FbSeff (lb-ft/ft)	Eleff (10^6lb-in^2/ft)	GAeff (10^6lb-in^2/ft)	Vs (lb/ft)	FbSeff (lb-ft/ft)	Eleff (10^6lb-in^2/ft)	GAeff (10^6lb-in^2/ft)	Vs (lb/ft
3 PLY (105V)	4.125	2,040	96	0.5	1,440	277	3.7	0.53	495
5 PLY (175V)	6.875	4,701	366	1.1	1.980	2,403	96	1.1	1,440
		DESIG	N VALUES F	OR IN-PLA		R OF CT	L PANELS		

CLT CONNECTIONS. SPLINES AND FASTENERS SHALL BE SHOWN IN THE STRUCTURAL DRAWINGS OR AS APPROVED BY THE SER. UNLESS OTHERWISE NOTED IN PLAN, CLT PANELS SHALL BE ORIENTED WITH EXTERIOR LAYERS PERPENDICULAR TO SUPPORTS FIELD NOTCHING AND BORING OF CLT PAMELS IS NOT ALLOWED UNLESS APPROVED BY SER.

290

9,700

22,400

14,400

24,000

PENETRATIONS: ALL PENETRATIONS THROUGH CLT PANELS WILL BE REVIEWED BY SER PRIOR TO FABRICATION. ALL PENETRATIONS ARE TO BE A MININUM OF (3) DIAMETERS APART AND NO GREATER THAN 4" IN DIAMETER

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15. PLYWOOD (PW) OR ORIENTED STRAND BOARD (OSB)

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195

270

- A. EACH PANEL SHALL BE IDENTIFIED WITH THE APPROPRIATE GRADE, TRADEMARK OF THE AMERICAN PLYWOOD ASSOCIATION, AND SHALL MEET THE REQUIREMENTS OF THE LATEST EDITION OF THE U.S. PRODUCT STANDARD PS-1. PLYWOOD GRADE SHALL CONFORM TO CD-X FOR PLYWOOD OR TYPE 2-M-W FOR ORIENTED STRAND BOARD, UNLESS OTHERWISE NOTED.
- B. WHERE PLYWOOD IS PERMANENTLY EXPOSED TO WEATHER, IT SHALL BE EXTERIOR TYPE. OTHERWISE, PANEL SHEATHING SHALL BE EXPOSURE 1. PLYWOOD TO BE CC GRADE AT LOCATIONS EXPOSED TO WEATHER; CC OR CD GRADE ELSEWHERE
- C. PANELS TO BE 5-PLY MINIMUM, EXCEPT 3/8" PANELS TO BE 3-PLY MINIMUM.
- D. PLYWOOD SHEETS AT FLOORS AND ROOFS SHALL BE LAID WITH FACE GRAIN PERPENDICULAR TO JOISTS AND RAFTERS. PLYWOOD AT FLOORS SHALL BE GLUED TO FRAMING BELOW (USE SOLVENT BASED GLUE COMPLYING WITH ASTM D3498 AND VOLATILE ORGANIC COMPOUND (VOC) LIMITS PER CALGREEN). LN-950 BY LIQUID NAILS OR APPROVED EQUIVALENT, UNLESS OTHERWISE SPECIFIED BY THE ARCHTIECT. PROVIDE RING-SHANK NAILS AT FLOOR AND ROOF SHEATHING.
- E. PLYWOOD SHEETS ON WALLS SHALL BE LAID WITH LONG DIMENSION VERTICAL. BLOCK ALL EDGES WITH A MINIMUM OF 3X BLOCK AND/MEMBERS. ALL NAILING SHALL HAVE 3/8 INCH EDGE DISTANCE FOR FRAMING. BLOCKING AND PLYWOOD EDGES. USE SMOOTH-SHANK NAILS FOR PLYWOOD WALL SHEATHING.
- F. STAPLES FOR PLYWOOD DIAPHRAGMS SHALL BE 14 GAGE ROUND SEMI-FLATTENED OR FLATTENED, PLAIN OR ZINC-COATED STEEL WIRE. WITH A NOMINAL CROWN WIDTH OF 7/16", DRIVEN BY PNEUMATIC OR MECHANICAL DEVICE.
- G. PROVIDE 1/8" GAP BETWEEN PANELS UNLESS OTHERWISE NOTED.
- H. PANELS SHALL HAVE THE FOLLOWING PROPERTIES UNLESS OTHERWISE NOTED
- 1) 3/8 INCH NOMINAL SHALL BE 3/8 INCH ACTUAL THICKNESS WITH 24/0
- SPAN RATING. 2) 1/2 INCH NOMINAL SHALL BE 15/32 INCH ACTUAL THICKNESS WITH 32/16
- SPAN RATING. 3) 5/8 INCH NOMINAL SHALL BE 19/32 INCH ACTUAL THICKNESS WITH 40/20

16. SELF-DRILLING SCREWS

SPAN RATING.

SELF-DRILLING SCREWS FOR WOOD AND WOOD TO STEEL CONNECTIONS SHALL BE SHOWN IN THE STRUCTURAL DRAWINGS FROM THE FOLLOWING APPROVED MANUFACTURERS.

APPROVED SELF-DRILLING SCREWS					
SCREW TYPE (CALL OUT)	MANUFACTURER	ICC REPORT			
SDS SERIES WOOD SCREWS (SDS)	SIMPSON STRONG-TIE	ESR-2236			
SDWS SERIES WOOD SCREWS (SPWS)	SIMPSON STRONG-TIE	ER-192			
SWG STRUCTURAL SCREWS (ASSY 3.0/VG)	MYTICON TIMBER CONNECTORS	ESR-3178 AND ESR-3179			

17. ROUGH CARPENTRY

- A. FOR SCHEDULE OF MINIMUM NAILING TABLE 2304.10.1 OF THE 2019 CBC/2018 IBC 16d VINYL COATED SINKERS MAY BE SUBSTITUTED FOR 16d BOX OR COMMON NAILS FOR ROUGH FRAMING. SINKERS SHALL NOT BE USED WITH METAL CONNECTORS.
- B. SILLS AND LEDGERS ON CONCRETE OR MASONRY SHALL BE PRESSURE-TREATED DOUGLAS FIR. SILLS AND LEDGERS SHALL BE FASTENED TO THE CONCRETE WITH A MINIMUM OF TWO FASTENERS PER PIECE AND A FASTENER NO FURTHER THAN 9 INCHES FROM END OF EACH PIECE, UNLESS OTHERWISE NOTED.

C. PLACE JOISTS WITH CROWN UP.

D. RE-TIGHTEN ALL BOLTS PRIOR TO CLOSING IN WALLS.

- E. WHEN METAL CONNECTORS, ANCHORS OR FASTENERS ITEMS ARE EXPOSED TO WEATHER AND/OR PRESSURE TREATED LUMBER THE METAL ITEMS ARE TO BE OF HOT DIPPED ZINC-COATED GALVANIZED STEEL, STAINLESS STEEL, SILICON BRONZE OR COPPER. THE COATING WEIGHTS FOR ZINC-COATED FASTENERS SHALL BE IN ACCORDANCE WITH ASTM A153. SEE ADDITIONAL COATING REQUIREMENTS AS NOTED IN THE PRESSURE TREATMENT SECTION.
- DOUBLE ALL JOISTS UNDER ALL PARALLEL PARTITIONS UNLESS NOTED OTHERWISE.
- G. BLOCK ALL JOISTS AT SUPPORTS AND UNDER ALL PARTITIONS WITH MINIMUM 2x SOLID BLOCKING. BLOCK AND BRIDGE ROOF JOISTS AT 10 FEET AND FLOOR JOISTS AT 8 FEET UNLESS OTHERWISE NOTED.
- H. 2x JOISTS SHALL BE SISTERED (VERTICAL NAIL LAMINATED) WITH SDWS 0.220x3 MIN. LENGTH AT 6" O.C. IN (2) ROWS STAGGERED UNLESS OTHERWISE NOTED.
- I. ALL POSTS LOCATED OVER WOOD WALLS SHALL HAVE A POST OF EQUAL OR GREATER SIZE LOCATED IN THE WALL DIRECTLY BELOW UNLESS OTHERWISE NOTED.
- THE STRUCTURAL DESIGN ASSUMES THAT ALL FLOORS AND ROOFS ARE CONSTRUCTED AND LOADED WITH FINISHES (OR EQUIVALENT WEIGHT) FOR A MINIMUM OF SEVEN (7) DAY PRIOR TO THE TIME OF DOOR AND WINDOW INSTALLATION.
- K. ALL TIMBER FASTENERS NOT SPECIFICALLY DETAILED ON THE DRAWINGS SHALL BE SIMPSON STRONG-TIE'S STANDARD FASTENERS OR APPROVED EQUIVALENT INSTALLER PER MANUFACTURER'S INSTALLATION INSTRUCTIONS. USP LUMBER CONNECTORS WITH REFERENCE NUMBERS FOR SUBSTITUTION MAY BE USED IN LIEU OF SIMPSON HARDWARE. ENGINEER MAY APPROVE OF OTHER SUBSTITUTIONS UPON THE FOLLOWING: 1) WRITTEN REQUEST FOR OTHER BRANDS
- 2) SUBMISSION OF MANUFACTURER'S TESTING REPORTS
- 3) REFERENCES TO PERTINENT DETAILS WHERE SUBSTITUTIONS ARE TO BE APPLIED.
- ALL STRUCTURAL WOOD WALLS SHALL BE FRAMED WITH 2x4 MINIMUM STUDS AT 16" ON CENTER UNLESS OTHERWISE NOTED.
- M. PRE-DRILL HOLES AS REQUIRED TO PREVENT SPLITTING OF WOOD.

18. PRESSURE TREATMENT

- A. ALL LUMBER EXPOSED TO WEATHER SHALL BE PRESSURE TREATED IN ACCORDANCE WITH A.W.P.A. STANDARD U1, WITH A PRESERVATIVE AND RETENTION SUITABLE FOR THE APPLICATION (SEE BELOW). ALL CUT ENDS SHALL ALSO BE FIELD TREATED WITH A PRESERVATIVE. AS AN ALTERNATE, CONTRACTOR MAY USE REDWOOD OF EQUIVALENT STRENGTH PROPERTIES AS THOSE SHOWN ABOVE, AND AN APPROVED PRIMER.
- THE FOLLOWING USE CATEGORIES SHALL BE REQUIRED BASED ON THE **APPLICATION:**
- 1) UC1 INTERIOR DRY
- 2) UC2 INTERIOR DAMP
- 3) UC3A EXTERIOR ABOVE GROUND PROTECTED UC3B – EXTERIOR ABOVE GROUND - UNPROTECTED
- 5) UC4A GROUND CONTACT, GENERAL USE
- UC4B GROUND CONTACT, HEAVY DUTY USE 7) UC4C – GROUND CONTACT, EXTREME DUTY
- 8) UC5A MARINE USE, NORTHERN WATERS
- B. ALL PLYWOOD EXPOSED TO WEATHER SHALL BE PRESSURE TREATED.
- C. WHEN METAL CONNECTOR, ANCHOR OR FASTENER ITEMS ARE IN CONTACT WITH PRESSURE TREATED LUMBER AND/OR CORROSIVE ENVIRONMENTS THE CONTRACTOR SHALL USE CORROSION RESISTANT METAL ITEMS AS NOTED
- 1) WHEN LUMBER IS TREATED WITH CHROMATED COPPER ARSENATE (CCA-C) OR DOT SODIUM ARSENATE (SBX) THE METAL ITEMS SHALL HAVE A MINIMUM G90 (0.90 OZ/SQFT) ZINC COATING OR ENGINEER APPROVED EQUIVALENT.
- 2) WHEN LUMBER IS TREATED WITH ALKALINE COPPER QUAT (ACQ-C OR ACQ-D), COPPER AZOLE (CBA-A OR CA-B) OR OTHER BORATE (NON-DOT) TREATMENT THE METAL ITEMS SHALL HAVE A MINIMUM G185
- (1.85 OZ/SQFT) ZINC COATING OR ENGINEER APPROVED EQUIVALENT WHEN LUMBER IS TREATED WITH OTHER TREATMENTS (NOT AMMONIACAL COPPER ZINC ARSENATE (ACZA) SEE 4 BELOW) OR IS EXPOSED TO CORROSIVE ENVIRONMENTS NOT LIST ABOVE THE METAL ITEMS SHALL BE
- TYPE 316L STAINLESS STEEL OR ENGINEER APPROVED EQUIVALENT. 4) AMMONIACAL COPPER ZINC ARSENATE (ACZA) IS NOT PERMITTED UNLESS APPROVED BY THE ENGINEER.
- 5) CONTRACTOR IS TO CONFIRM LUMBER PRESSURE TREATMENT TYPE PRIOR TO PURCHASE OF METAL ITEMS.
- AS AN ALTERNATIVE, FOR THE SITUATION WHEN THE BASE OF A HOLDOWN IS IN CONTACT WITH A PRESSURE TREATED SILL PLATE THE CONTRACTOR CAN PROVIDE A PRESSURE TREATMENT BARRIER BETWEEN THE BASE OF THE HOLDOWN AND THE SILL PLATE.

19. CONCRETE UNIT MASONRY

- A. THE COMPRESSIVE STRENGTH OF THE MASONRY ASSEMBLY, F'm SHALL BE 2,500 PSI.
- CONCRETE BLOCK MASONRY UNITS SHALL CONFORM TO ASTM C90-16a AND SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3,750 PSI.
- C. ALL MASONRY UNITS SHALL BE CHANNEL BLOCKS, UNLESS OTHERWISE NOTED
- D. GROUT SHALL ATTAIN A MINIMUM COMPRESSIVE STRENGTH OF 2,500 PSI AT 28 DAYS.
- MORTAR SHALL CONFORM TO ASTM C270-14a, TYPE M. MORTAR SHALL ATTAIN A MINIMUM COMPRESSIVE STRENGTH OF 2.500 PSI AT 28 DAYS.
- F. GROUT SHALL CONFORM TO ASTM C476.
- G. ALL CELLS SHALL BE FILLED SOLID WITH GROUT AND REINFORCED AS SHOWN IN SPECIFIC DETAILS UNLESS OTHERWISE NOTED.
- H. PROVIDE CLEAN-OUT HOLES FOR HIGH LIFT GROUT APPLICATIONS AND GROUT POUR OVER 5 FEET. CLEAN-OUT HOLES, WHEN REQUIRED, ARE TO BE PROVIDED AT EVERY VERTICAL BAR, 32 INCHES MAXIMUM SPACING.
- LAP SPLICE ALL REINFORCING BARS IN MASONRY A MINIMUM OF 48 BAR DIAMETERS, UNLESS OTHERWISE NOTED. STAGGER ALL SPLICES FOR A MINIMUM OF 24 INCHES OR LAP LENGTH, WHICHEVER IS LONGER. IN LIEU OF LAP SPLICES. REBAR COUPLERS MAY BE USED WITH THE WRITTEN APPROVAL OF THE ENGINEER OF RECORD ON A CASE BY CASE BASIS.

20. MECHANICAL ANCHORS

- A. EXPANSION ANCHORS INTO CONCRETE SHALL BE a. HILTI KB-TZ.
- b. SIMPSON STRONG-BOLT 2. INSTALL ANCHORS IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS.
- B. SCREW ANCHORS INTO CONCRETE SHALL BE:
- a. HILTI KH-EZ,
- b. SIMPSON TITEN HD, INSTALL SCREWS IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS.
- C. PROVIDE STAINLESS (AISI 316) STEEL FASTENERS FOR EXTERIOR USE OR WHEN EXPOSED TO WEATHER OR IN CHEMICALLY CORROSIVE ENVIRONMENTS. PROVIDE ZINC COATED OR GALVANIZED CARBON STEEL ANCHORS AT OTHER LOCATIONS. UNLESS OTHERWISE NOTED. WHERE STAINLESS STEEL FASTENERS ARE USED IN CONJUNCTION WITH GALVANIZED OR OTHER DISSIMILAR BASE METALS, PROVIDE ELECTRICAL ISOLATION AS NOTED ON THE DRAWINGS. NOTIFY THE ENGINEER FOR CLARIFICATION IF NO ELECTRICAL ISOLATION IS SPECIFIED.

- D. IF REINFORCEMENT IS ENCOUNTERED DURING DRILLING, ABANDON AND SHIFT ANCHOR DIAMETERS OR 1 INCH, WHICHEVER IS LARGER, OF SOUND CONCRETE BETWEEN THE DOWEL AND THE ABANDONED HOLE. FILL THE ABANDONED HOLE WITH NON-SHRINK GROUT. DO NOT CUT EXISTING REINFORCEMENT. IF THE ANCHOR OR DOWEL MAY NOT BE SHIFTED AS NOTED ABOVE, THE ENGINEER WILL DETERMINE A NEW LOCATION.
- E. LOCATE REINFORCEMENT AND CONFIRM FINAL ANCHOR LOCATIONS PRIOR TO FABRICATING PLATES, MEMBERS, OR OTHER STEEL ASSEMBLIES ATTACHED WITH MECHANICAL ANCHORS.

21. EPOXY GROUTING OF DOWELS, REBAR AND ANCHOR BOLTS

- A. INSTALLATION OF POST-INSTALLED DOWELS, REBAR AND ANCHOR BOLTS (EPOXY ANCHORS) SHALL BE IN ACCORDANCE WITH THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS (MPII). WHERE THERE IS A CONFLICT BETWEEN THESE NOTES AND THE MPII, SEE MPII FOR CLARIFICATION.
- B. EPOXY ANCHORS SHALL MEET THE REQUIREMENTS OF ACI 355.4 AND THE FOLLOWING INSTALLATION REQUIREMENTS, UNLESS OTHERWISE NOTED.
- 1) MINIMUM AGE OF CONCRETE: 21 DAYS 2) CONCRETE TEMPERATURE RANGE: 50-80 DEGREES FAHRENHEIT MOISTURE CONDITION OF CONCRETE: DRY
- C. EPOXY GROUTING WILL BE USED IN ALL LOCATIONS WHERE EITHER ALL-THREAD ROD OR REBAR ARE BEING EMBEDDED INTO EXISTING CONCRETE, CMU, OR BRICK.
- D. IN CONCRETE, HOLES SHALL BE DRILLED WITH ROTARY HAMMER UNLESS NOTED OTHERWISE.
- E. EPOXY GROUT FOR DOWNWARD HOLES SHALL BE EITHER NON-SAG OR LIQUID TYPE, NORMAL SET. HORIZONTAL OR OVERHEAD HOLES SHALL BE NON-SAG TYPE. FOR OVERHEAD APPLICATIONS A PISTON PLUG SHALL BE USED.
- F. UNLESS OTHERWISE NOTED, EPOXY TYPES SHALL BE AS FOLLOWS: FOR DOWELS AND REBAR IN CONCRETE, EPOXY SHALL BE: a. HILTI HIT-RE 500 V3.
- FOR ANCHOR BOLTS IN CONCRETE, EPOXY SHALL BE a. SIMPSON SET-XP. b. HILTI HIT-HY 200,
- FOR CONCRETE MASONRY UNITS (CMU), EPOXY SHALL BE SIMPSON SET ALTERNATES WILL BE CONSIDERED UPON REQUEST AND SUBMISSION OF PRODUCT EVALUATION REPORT IN ACCORDANCE WITH ACI 355.4.
- 1) WHEN INSTALLING ANCHORS, USE CARE AND CAUTION TO AVOID CUTTING OR DAMAGING THE EXISTING REINFORCING BARS OR POST TENSIONING TENDONS. IN POST TENSION ELEMENTS THE CONTRACTOR SHALL SCAN
- 2) IF REINFORCEMENT IS ENCOUNTERED DURING DRILLING, ABANDON AND SHIFT THE HOLE LOCATION TO AVOID THE REINFORCEMENT. PROVIDE A MINIMUM OF 2 ANCHOR DIAMETERS OR 1 INCH. WHICHEVER IS LARGER.
- A NEW LOCATION. ATTACHED WITH ANCHORS.

THE HOLE LOCATION TO AVOID THE REINFORCEMENT. PROVIDE A MINIMUM OF 2

PRIOR TO LOCATE THE EXISTING TENDONS PRIOR TO INSTALLING THE ANCHOR OF SOUND CONCRETE BETWEEN THE DOWEL AND THE ABANDONED HOLE. FILL THE ABANDONED HOLE WITH NON-SHRINK GROUT. IF THE ANCHOR OR DOWEL MAY NOT BE SHIFTED AS NOTED, THE ENGINEER WILL DETERMINE

3) LOCATE EXISTING REINFORCEMENT AND CONFIRM FINAL ANCHOR LOCATIONS PRIOR TO FABRICATING PLATES, MEMBERS, OR OTHER STEEL ASSEMBLIES



S-002





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2 PLAN CHECK COMMENTS

BLDG. DEPT.	SUBMITTAL
JOB NO.	20149.10
DRAWN	CJ
DATE	4/20/2021
SCALE	As indicated
FILENAME	
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MINIMUM TEST AND SPECIAL INSPECTIONS OF 2 **CONCRETE CONSTRUCTION** N.T.S.

a. WHERE APPLICABLE, SEE ALSO SECTION 1705.12 (SPECIAL INSPECTIONS FOR SEISMIC RESISTANCE).

UNLESS NOTED OTHERWISE.

1. INSPECT REINFORCEMENT, INCLUDING PRESTRESSING TENDONS, AND PLACEMENT.	-	x	ACI 318: CH. 20, 25.2, 25.3, 26.6.1-26.6.3	1908.4
2. REINFORCING BAR WELDING:		L		
a. VERIFY WELDABILITY OF REINFORCING BARS OTHER THAN ASTM A706.	-	x		
b. INSPECT SINGLE-PASS FILLET WELDS, MAXIMUM 5/16".	-	x	ACI 318: 26.6.4	-
c. INSPECT ALL OTHER WELDS.	х	-		
3. INSPECT ANCHORS CAST IN CONCRETE.	-	х	ACI 318: 17.8.2	-
4. INSPECTION OF ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS .				-
a. ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS.	x	-	ACI 318: 17.8.2.4	-
b.MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED IN 4.a.	-	x	ACI 318: 17.8.2	-
5. VERIFY USE OF REQUIRED MIX DESIGN.	-	x	ACI 318: CH.19, 26.4.3, 26.4.4	1904.1, 1904 1908.2, 1908
6. PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE.	x	-	ASTM C172, ASTM C31, ACI 318: 26.4.5, 26.12	1908.10
7. INSPECTION OF CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES.	x	-	ACI 318: 26.5	1908.6, 1908.7,
8. VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES	- 3.	x	ACI 318: 26.5.3-26.5.5	1908.0
9. INSPECT PRESTRESSED CONCRETE FOR:				
a. APPLICATION OF PRESTRESSING FORCES.	x	-	ACI 318: 26.10	-
b. GROUTING OF BONDED PRESTRESSING TENDONS.	x	-	ACI 318: 26.10	-
10. INSPECT ERECTION OF PRECAST CONCRETE MEMBERS.	-	x	ACI 318: 26.9	-
11. VERIFY IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POST-TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS.	-	x	ACI 318: 26.11.2	-
12. INSPECT FORMWORK FOR SHAPE LOCATION, AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.	-	x	ACI 318: 26.11.1.2(b)	-

T STATEMENT OF SPECIAL INSPECTIONS S-003

REQUIRED SPECIAL INSPECTIONS AND TESTS OF CONCRETE CONSTRUCTION (2019 CBC TABLE 1705.3 AND ACI 318-14)²

WHERE THE EFFECTIVE PRESTRESS IN THE CONCRETE IS LESS THAN 150 PSI.

ASSURANCE REQUIREMENTS, INCLUDING THE SPECIAL INSPECTION TABLES SHOWN HEREIN.

c. CONCRETE PATIOS, DRIVEWAYS AND SIDEWALKS, ON GRADE.

N.T.S.

- COMPONENT. THE CONTRACTOR'S STATEMENT OF RESPONSIBILITY SHALL CONTAIN ACKNOWLEDGMENT OF AWARENESS OF THE SPECIAL REQUIREMENTS CONTAINED IN THIS STATEMENT OF SPECIAL INSPECTIONS. 10. STEEL CONSTRUCTION: SPECIAL INSPECTIONS FOR STEEL ELEMENTS OF BUILDINGS AND STRUCTURES SHALL BE AS REQUIRED BY SECTION 1705.2 OF THE CODE AND IN ACCORDANCE WITH THE QUALITY ASSURANCE INSPECTION REQUIREMENTS OF AISC 360-10, INCLUDING THE SPECIAL INSPECTION TABLE SHOWN HEREIN. SEE ALSO REQUIREMENTS NOTED FOR SEISMIC AND WIND RESISTANCE OF INSPECTION NOTES #8 AND #9. 11. CONCRETE CONSTRUCTION: SPECIAL INSPECTIONS AND VERIFICATIONS FOR CONCRETE CONSTRUCTION SHALL BE AS REQUIRED BY SECTION 1705.3 OF THE CODE, INCLUDING THE SPECIAL INSPECTION TABLE SHOWN HEREIN. CONCRETE SPECIAL INSPECTIONS AND TESTS ARE NOT REQUIRED FOR:

- RESPONSIBILITY TO THE BUILDING OFFICIAL AND THE OWNER PRIOR TO THE COMMENCEMENT OF WORK ON THE SYSTEM OR
- SEISMIC SYSTEM, OR A WIND OR SEISMIC RESISTING COMPONENT LISTED ABOVE SHALL SUBMIT A WRITTEN STATEMENT OF

- 9. EACH CONTRACTOR RESPONSIBLE FOR THE CONSTRUCTION OF A MAIN WIND OR SEISMIC FORCE RESISTING SYSTEM, DESIGNATED
- SEE THE ABOVE-REFERENCED CODE SECTIONS FOR ADDITIONAL SPECIAL INSPECTION AND TEST REQUIREMENTS FOR STRUCTURAL STEEL, STRUCTURAL WOOD, COLD-FORMED STEEL LIGHT-FRAME CONSTRUCTION, DESIGNATED SEISMIC SYSTEMS, ARCHITECTURAL COMPONENTS, MEP COMPONENTS, STORAGE RACKS, SEISMIC ISOLATIONS SYSTEMS, AND COLD-FORMED STEEL SPECIAL BOLTED MOMENT FRAMES.
- FORCE RESISTING COMPONENT WHEN APPLICABLE AND AS PER SECTIONS 1705.12 & 1705.13 OF THE CODE. a. DESIGNATED SEISMIC SYSTEM/SEISMIC FORCE RESISTING SYSTEM: WRITE IN APPLICABLE SYSTEM(S) OR "N/A".
- CONFORMANCE WITH THE REQUIREMENTS OF THE STRUCTURAL DRAWINGS. 8. SPECIAL INSPECTIONS AND TESTS FOR SEISMIC RESISTANCE SHALL BE PERFORMED FOR THE DESIGNATED SEISMIC SYSTEM/SEISMIC
- SEPARATE FROM THE SPECIAL INSPECTION REPORTS. 7. SPECIAL INSPECTION REPORTS SHALL BE SENT TO THE ENGINEER AT THE TIME OF COMPLETION FOR REVIEW OF
- NON-CONFORMING WORK. THIS NOTIFICATION SHALL SPECIFICALLY ADDRESS THE NON-CONFORMING WORK AND SHALL BE
- TESTS. 6. IF INITIAL TESTS OR INSPECTIONS MADE BY THE OWNER'S TESTING OR INSPECTION AGENCY REVEAL THAT ANY PORTION OF THE WORK DOES NOT COMPLY WITH THE CONTRACT DOCUMENTS, ADDITIONAL TESTS, INSPECTIONS, AND NECESSARY REPAIRS SHALL BE MADE AT THE CONTRACTOR'S EXPENSE. CONTRACTOR SHALL NOTIFY THE ENGINEER AND OWNER IMMEDIATELY OF
- 5. THE CONSTRUCTION OR WORK FOR WHICH SPECIAL INSPECTION OR TESTING IS REQUIRED SHALL REMAIN ACCESSIBLE AND EXPOSED FOR SPECIAL INSPECTION OR TESTING PURPOSES UNTIL COMPLETION OF THE REQUIRED SPECIAL INSPECTIONS OR
- 3. THE SPECIAL INSPECTION AND/OR TESTING AGENCY SHALL KEEP RECORDS AND SUBMIT SPECIAL INSPECTION AND TEST REPORTS TO THE BUILDING OFFICIAL AND THE STRUCTURAL ENGINEER OF RECORD IN ACCORDANCE WITH SECTIONS 1704.2.4 AND 1704.5 OF THE CODE AND JURISDICTION-SPECIFIC REQUIREMENTS. 4. THE CONTRACTOR SHALL NOTIFY THE TESTING LAB A MINIMUM OF 48 HOURS PRIOR TO TIME OF INSPECTION.
- 2. THE OWNER SHALL BE RESPONSIBLE FOR RETAINING THE SPECIAL INSPECTION AND/OR TESTING AGENCY.

STATEMENT OF SPECIAL INSPECTIONS

EARTH OR ROCK.

VERIFICATION AND INSPECTION

a. ISOLATED SPREAD FOOTINGS OF BUILDINGS 3 STORIES OR LESS ABOVE GRADE PLANE THAT ARE FULLY SUPPORTED ON b. NONSTRUCTURAL CONCRETE SLABS SUPPORTED DIRECTLY ON THE GROUND, INCLUDING PRESTRESSED SLABS ON GRADE

12. MASONRY CONSTRUCTION: SPECIAL INSPECTIONS AND VERIFICATIONS FOR MASONRY CONSTRUCTION SHALL BE AS REQUIRED BY SECTION 1705.4 OF THE CODE AND IN ACCORDANCE WITH TMS 402/ACI 530/ASCE 5 AND TMS 602/ACI 530.1/ASCE 6 QUALITY

13. WOOD CONSTRUCTION: SPECIAL INSPECTIONS FOR WOOD CONSTRUCTION SHALL BE AS REQUIRED BY SECTION 1705.5 OF THE CODE. SEE ALSO REQUIREMENTS NOTED FOR SEISMIC AND WIND RESISTANCE OF INSPECTION NOTES #8 AND #9.

14. SOILS: SPECIAL INSPECTIONS FOR EXISTING SOIL CONDITIONS, FILL PLACEMENT, AND LOAD BEARING REQUIREMENTS SHALL BE AS REQUIRED BY SECTIONS 1705.6 THROUGH 1705.9 OF THE CODE, INCLUDING THE SPECIAL INSPECTION TABLES SHOWN HEREIN.

> CONTINUOUS PERIODIC REFERENCED STANDARD CBC REFERENCE 1908.4 -

> > -

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, 1904.2, 8.2, 1908.3

1908.10

1908.7, 1908.8

1908.0

-

b. SPECIFIC REQUIREMENTS FOR SPECIAL INSPECTION SHALL BE INCLUDED IN THE RESEARCH REPORT FOR THE ANCHOR ISSUED BY AN APPROVED SOURCE IN ACCORDANCE WITH ACI 318-14 SECTION 17.8.2, OR OTHER QUALIFICATION PROCEDURES. WHERE SPECIFIC REQUIREMENTS ARE NOT PROVIDED, SPECIAL INSPECTION REQUIREMENTS SHALL BE SPECIFIED BY THE REGISTERED DESIGN PROFESSIONAL AND SHALL BE APPROVED BY THE BUILDING OFFICIAL PRIOR TO THE COMMENCEMENT OF THE WORK. SPECIAL INSPECTIONS FOR EPOXY ADHESIVE ANCHORS SHALL BE CONTINUOUS

REQUIRED SPECIAL INSPECTIONS AND TESTS OF SOILS (2019 CBC TABLE 1705.6)					
TYPE	CONTINUOUS	PERIODIC			
1. VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.	-	Х			
2. VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.	-	х			
3. PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS.	-	Х			
4. VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL.	х	-			
5. PRIOR TO PLACEMENT OF COMPACTED FILL, INSPECT SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.	-	Х			



MINIMUM TESTS AND SPECIAL INSPECTIONS OF SOILS N.T.S.

MINIMUM TESTS AND SPECIAL INSPECTION OF MASC LEVEL B TESTS AND SPECIAL INSPECTIONS FOR RISP	NRY CONSTRUC [®] (CATEGORY I, II,	TION (2019 C AND III PER A	BC SECTION 1705.4) ACI 530.1-13 TABLE 4
VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC	REFERENCED STANDARD
1. TESTS: VERIFICATION OF SLUMP FLOW AND VISUAL STABILITY INDEX (VSI) AS DELIVERED TO THE PROJECT SITE FOR SELF-CONSOLIDATING GROUT.	х	-	ACI 530.1 ART. 1.5B.1.b.3
2. TESTS: VERIFICATION OF fm AND fAAC PRIOR TO CONSTRUCTION EXCEPT WHERE SPECIFICALLY EXEMPTED BY THE CODE.	Х	-	ACI 530.1 ART. 1.4B
3. VERIFY COMPLIANCE WITH THE APPROVED SUBMITTALS.	-	x	ACI 530.1 ART. 1.5
4. AS MASONRY CONSTRUCTION BEGINS, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE:			
a. PROPORTIONS OF SITE-PREPARED MORTAR.	-	x	ACI 530.1 ART. 2.1, 2.6A
b. CONSTRUCTION OF MORTAR JOINTS.	-	x	ACI 530.1 ART. 3.3B
c. GRADE AND SIZE OF PRESTRESSING TENDONS AND ANCHORAGES.	-	х	ACI 530.1 ART. 2.4B, 2.4H
d.LOCATION OF REINFORCEMENT, CONNECTORS, AND PRESTRESSING TENDONS AND ANCHORAGES.	-	х	ACI 530.1 ART. 3.4, 3.6A
e.PRESTRESSING TECHNIQUE.	-	x	ACI 530.1 ART. 3.6B
f. PROPERTIES OF THIN-BED MORTAR FOR AAC MASONRY.	x ^(a)	x ^(b)	ACI 530.1 ART. 2.1C
5. PRIOR TO GROUTING, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE:		1	
a. GROUT SPACE.	_	x	ACI 530.1 ART. 3.2D, 3.2F
b. GRADE, TYPE, AND SIZE OF REINFORCEMENT AND ANCHOR BOLTS, AND PRESTRESSING TENDONS AND ANCHORAGES.	-	x	ACI 530 SEC. 6.1; ACI 530.1 ART. 2.4, 3.4
c. PLACEMENT OF REINFORCEMENT, CONNECTORS, AND PRESTRESSING TENDONS AND ANCHORAGES.	-	х	ACI 530 SEC. 6.1, 6.2.1, 6.2.6, 6.2.7; ACI 530.1 ART. 3.2E, 3.4, 3.6A
d. PROPORTIONS OF SITE-PREPARED GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS.	-	х	ACI 530.1 ART. 2.6B, 2.4G.1.b
e. CONSTRUCTION OF MORTAR JOINTS.	-	х	ACI 530.1 ART. 3.3B
6. VERIFY DURING CONSTRUCTION:			
a. SIZE AND LOCATION OF STRUCTURAL ELEMENTS.	-	x	ACI 530.1 ART. 3.3F
b. TYPE, SIZE, AND LOCATION OF ANCHORS, INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES, OR OTHER CONSTRUCTION.	-	x	ACI 530 SEC. 1.2.1(e), 6.1.4.3, 6.2.1
c. WELDING OF REINFORCEMENT.	х	-	ACI 530 SEC. 8.1.6.7.2, 9.3.3.4(c) 11.3.3.4(b)
d. PREPARATION, CONSTRUCTION, AND PROTECTION OF MASONRY DURING COLD WEATHER (TEMPERATURE BELOW 40°F) OR HOT WEATHER (TEMPERATURE ABOVE 90°F).	F _	x	ACI 530.1 ART. 1.8C, 1.8D
e. APPLICATION AND MEASUREMENT OF PRESTRESSING FORCE.	З X	-	ACI 530.1 ART. 3.6B
f. PLACEMENT OF GROUT AND PRESTRESSING GROU FOR BONDED TENDONS IS IN COMPLIANCE.	тх	-	ACI 530.1 ART. 3.5, 3.6C
g. PLACEMENT OF AAC MASONRY UNITS AND CONSTRUCTION OF THIN-BED MORTAR JOINTS.	x ^(a)	x ^(b)	ACI 530.1 ART. 3.3B.9, 3.3F.1.b
7. OBSERVE PREPARATION OF GROUT SPECIMENS, MORTAR SPECIMENS, AND/OR PRIS.M.S	-	х	ACI 530.1 ART. 1.4B.2.a.3, 1.4B.2.b.3, 1.4B.2.c.3, 1.4B.3, 1.4B.4

a. REQUIRED FOR THE FIRST 5,000 SQUARE FEET OF AAC MASONRY. b. REQUIRED AFTER THE FIRST 5,000 SQUARE FEET OF AAC MASONRY.



N.T.S.

TESTING FOR SEISMIC RESISTANCE (2019 CBC SECTION 1705.13)				
TESTING	REFERENCED STANDARD			
3. DESIGNATED SEISMIC SYSTEMS: REVIEW CERTIFICATE OF COMPLIANCE FOR ELEMENTS OF THE DESIGNATED SEISMIC SYSTEM (WHERE NOTED ON THESE DRAWINGS) FOR CONFORMANCE WITH ASCE 7-16 SECTION 13.2.2.	CBC SEC. 1705.13.3			

EST FOR SEISMIC RESISTANCE

N.T.S.

REQUIRED VERIFICATION AND INSPECTION FOR SEISMIC RESISTANCE (2019 CBC SECTION 1705.12)					
VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC ^a	REFERENCED STANDARD		
2. STRUCTURAL WOOD SPECIAL INSPECTIONS FOR SEISMIC RESISTANCE:			CBC SEC. 1705.12.2		
a. INSPECTION OF FIELD GLUING OPERATIONS OF ELEMENTS OF THE SEISMIC-FORCE RESISTING SYSTEM.	x	-			
b. INSPECTION OF NAILING, BOLTING, ANCHORING AND OTHER FASTENING OF COMPONENTS WITHIN THE SEISMIC-FORCE RESISTING SYSTEM, INCLUDING WOOD SHEAR WALLS*, WOOD SHEAR PANELS*, WOOD DIAPHRAGMS*, DRAG STRUTS, AND HOLD-DOWNS.	-	x	* SPECIAL INSPECTIONS NOT REQUIRED WHERE FASTER SPACING OF SHEATHING IS MORE THAN 4" O.C.		

a. "O" INDICATES AN ACTIVITY THAT IS EITHER A ONE-TIME ACTIVITY OR ONE WHOSE FREQUENCY IS ON A RANDOM BASIS OR IS DEFINED IN SOME OTHER MANNER (SEE REFERENCED CODE SECTION).

MINIMUM INSPECTION FOR SEISMIC RESISTANCE 6 S-003 N.T.S.





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S-003





OPP. FACE

(1) #5 x 6'-0" @ EA. CORNER





8"-16" >16"

1" = 1'-0"

2 1/2"

		F	REINFORCING S	TEEL SCHEDULE	41 	
WALL/SLAB	TYP.	REINF. STEEL	INFO	REQUIRED AI	REQUIRED ADDN. REINF. @ CONT. JOINT	
THICKNESS	SIZE	SPACING (IN)	# OF CURTAINS	SIZE	SPACING (IN)	# OF CURTAINS
6"	#4	12	1	#4	12	1
8"	#5	12	1	#5	12	1
12"	#5	12	2	#5	10	1
12"	#8	12	2	#8	24	1
18"	#6	12	2	#6	8	1
18"	#8	12	2	#8	12	1
18"	#10	12	2	#10	12	1
18"	#11	12	2	#11	24	1
24"	#7	12	2	#7	8	1
24"	#8	12	3	#8	12	1
24"	#10	12	3	#10	12	1
36"	#10	12	3	#10	12	1

ADD. CONT. JOINT REINF. STEEL SCHEDULE (13) S-011



1" = 1'-0"

TYPICAL SLAB ON GRADE

NOTES: 1.AT EDGE OF SLAB LOCATIONS, HOOK REINF. INTO OPPOSITE FACE OF WALL OR BEAM

TYP. CONCRETE FLOOR OPENINGS



BAR SPACING FOR BARS SPLICED WITH A





1. SEE DRAWINGS BY OTHERS FOR HEIGHT, SIZE, CHAMFERS & LOCATION OF CONCRETE PADS. 2. COORDINATE REINF. LOCATIONS TO AVOID INTERFERENCE WITH INSTALLATION OF EXP. ANCHORS IF USED.

SLAB HOUSEKEEPING PAD S-011

NOTES:

N.T.S.



NOTES: 1. THIS TABLE CONTAINS MIN. LENGTHS FOR LAP SPLICES & BAR DEVELOPMENT NOT OTHERWISE SPECIFIEDON THESE DRAWINGS THESE LENGTHS MAY BE REDUCED IN CERTAIN SITUATIONS, SUBJECT TO PRIOR REVIEW & APPROVAL OF THE ENGINEER.

SPLICE LENGTHS ARE FOR NORMAL WEIGHT CONC. W/ GRADE 60 REINF. MULTIPLY SPLICE LENGTHS BY 1.33 FOR LIGHTWEIGHT CONC.

SPLICE LENGTHS ARE FOR UNCOATED BARS. DIVIDE LENGTHS IN TABLE BY 1.3 TO OBTAIN SINGLE STRAIGHT BAR, DEVELOPMENT LENGTHS IN

CONCRETE 6. USE "TOP" FOR WALL BOUNDARIES & WHEN MORE THAN 12" OF FRESH CONC. IS PLACED BELOW SPLICE, "OTHER" FOR ALL OTHER SITUATIONS.

'S' = SPACING. 8. PROVIDE MIN. COVER PER GENERAL NOTES, BUT NOT LESS THAN 1x BAR DIAMETER.



N.T.S.

N.T.S.

3"

6"

MIN. BAR SPACING: 1 1/2" OR 1 1/2"d

MAX. BAR SPACING: Ls/5 OR 6" WHICHEVER IS LESS.

NON-CONTACT LAP

1" = 1'-0"





NORMAL WEIGHT CONCRETE. MULTIPLY HOOK DEVELOPMENT LENGTH BY 1.33 FOR LIGHTWEIGHT CONCRETE. 5. DO NOT FIELD BEND REINFORCEMENT PARTALLY EMBEDDED IN CONCRETE.

(1) S-011

STANDARD HOOK DIM. / DEVELOPMENT SCHED.





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2 FOOTING REINFORCING AT CORNER AND S-012 INTERSECTION



3/8" = 1'-0"





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6 TOP PLATE SPLICE @ BEARING OR SHEARWALLS S-021

(25) 16d NAILS, STAG.

E/S OF SPLICE

+ +

+ + + + + +

UPPER PLATE SPLICE

TYP.

PLAN

FC

TYP. $\downarrow \neq TYP$.

i + | +

+ +

EQ

WALL STUD SCHEDULE





4. EXTERIOR WALL STUDS SHALL BE 2x6 MIN. 5. SEE ARCHITECTURAL DRAWINGS FOR WALL THICKNESS AT INTERIOR BEARING/SHEAR WALLS 6. ALL WALLS TO BE PER SCHEDULE U.O.N. ON PLAN OR DETAILS

5

S-021





LENGTH OF SPLICE

4'-0" MIN., TYP.

+ +

TYPICAL BEARING / SHEAR WALL FRAMING

N.T.S.



310

460

600

770

[6]

[4]

S-021 /

NOTES: 1. USE ½" CDX PLYWD.

10d @ 6" O.C.

10d @ 4" O.C.

10d @ 3" O.C.

10d @ 2" O.C.

SHORTS (2 1/8" MIN. LENGTH) W/ FULL HEADS.

RECEIVING EDGE NAILING CAN BE 2x.

N.T.S.

FDN. ANCHOR

SPACING. SEE

48" O.C.

48" O.C.

32" O.C.

24" O.C.

(_e)NOTE 4

- ANCHOR BOLTS SEE

S.W. SCH.

16" O.C.

12" O.C.

8" O.C.

8" O.C.

1 OOTT EIRT EAN	L>			5000113	
	 		<u> </u>	5	
CONTINUOUS WALL WHERE OCCURS	E.	N.		- HOLDOWN PER PLAN WHERE OCCURS	
	<u>Sł</u>	IEAR WALL II	NTERSECTION		
3 HOLDO	WN DETA	LS - PLA	N VIEW		
S-021				1"	= 1'-0"
EDGE NAILING AT HORIZ & VERT. EDGES STAGGE NAILS W/ NAILS FROM ADJACENT PANELS —	z. ER			 STUD & BLKG. SIZE AT PANEL EDGES SEE S.W. SCH. 	S
LEAVE 1/16" GAP BETWEEN PANEL EDGES, TYP BLKG. CENTERED ON HORIZ. PLYWOOD JOINTS VERT. PLYWOOD JOINTS TO BE CENTERED ON STUDS END POST & HOLDOWN PER HOLDOWN SCHEDULE WHERE OCCURS				TYP. STUE FIELD NAII SEE S.W.)S W/ LING SCH.
SILL PLATE PER S.W. SCH. SILL PLATE SHALL BE PTDF WHEN IN				— MIN. PANEL SIZE 24	I" SQ.

E.N. E.N. SW PER PLAN WHERE OCCURS



SHEAR WALL SCHEDULE

CLOSER THAN 8" O.C. RIM OR RIM BLOCKING SHALL BE 3½" MIN. WIDTH AND FASTENERS SHALL BE

STAGGERED. SDS 1/4 x 6 MAY BE USED IN LIEU OF SDWS 0.220 x 6 AT CONTRACTOR'S DISCRETION.

SIMPSON SET-XP EPOXY MAY BE USED. ANCHORS SHALL HAVE A MIN. EMBEDMENT OF 7", A MIN. EDGE

BOTH SIDES, ANCHOR BOLTS SHALL BE STAGGERED. A.B. & WASHER SHALL BE HOT DIPPED GALVANIZED.

5. SILL CONNECTION IS FOR WOOD TO WOOD CONNECTION ONLY, TYP. BTWN. FLOORS. WHERE SPACING IS

16" O.C.

12" O.C.

8" O.C.

8" O.C.





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NAILING SCHEDULE	
INECTION	NAILING
DER, TOE NAIL	(3) 8d
OE NAIL E/E	(2) 8d
ESS TO EA. JOIST, FACE NAIL	(2) 8d
JBFLOOR TO EA. JOIST, FACE	(3) 8d
T OR GIRDER, BLIND & FACE	(2) 16d
OR BLOCKING, FACE NAIL , AT BRACED WALL PANELS	16d @ 16" O.C. (3) 16d @ 16" O.C.
END NAIL	(2) 16d
	(4) 8d TOE NAIL OR (2) 16d END NAIL
NAIL	16d @ 24" O.C.
, FACE NAIL LAP SPLICE (PARTITION)	16d @ 16" O.C. (8) 16d
JOISTS OR RAFTERS TO TOP	(3) 8d
ATE, TOE NAIL	8d @ 16" O.C.
D INTERSECTIONS, FACE NAIL	(2) 16d
R, TWO PIECES	16d @ 16" O.C. ALONG EACH EDGE
_ATE, TOE NAIL	(3) 8d
R TO STUD, TOE NAIL	(4) 8d
OVER PARTITIONS, FACE NAIL	(3) 16d
ARALLEL RAFTERS, FACE NAIL	(3) 16d MIN. SEE 2010 CBC TABLE 2308.10.4.1
DE NAIL	(3) 8d
O EA. STUD & PLATE, FACE	(2) 8d
LESS TO EA. BEARING, FACE	(2) 8d
EATHING TO EA. BEARING,	(3) 8d
JDS	16d @ 24" O.C.
EAMS	20d @ 32" O.C. FACE NAIL T&B STAGG. ON OPP. SIDES & (2) 20d FACE NAIL AT ENDS AND SPLICES
	16d @ EACH BEARING
R, FACE NAIL	(3) 10d
	(3) 10d TOE NAIL (2) 16d FACE NAIL
RIDGE BEAM	(2) 16d TOE NAIL (2) 16d FACE NAIL
FACE NAIL	(3) 16d
NAIL AT EACH JOIST	(3) 16d
PANELS SUBFLOOR, ROOF & FRAMING)	10d
AMING)	8d
ING	8d
	6d

(5)

∖S-022 ∕





OPENING REINF. @ WOOD S.W.

EXTERIOR WALLS & INTERIOR BEARING WALLS

MAX. OPNG	MIN. HDR.	SUPPORTING ROOF		SUPPORTING ROOF +1 FLR.		SUPPORTING ROOF +2 FLR.	
WIDTH	SIZE	BEARING STUD	KING STUD	BEARING STUD	KING STUD	BEARING STUD	KING STUD
3'-0"	6x6	2x6	2x6	2x6	2x6	2x6	2x6
5'-0"	6x8	2x6.	2x6.	2x6.	2x6.	2x6.	2 - 2x6.
7'-0"	6x10	2x6.	2 - 2x6.	2 - 2x6.	2x6.	2 - 2x6.	2 - 2x6.
9'-0"	6x12	2 - 2x6.	2 - 2x6.	2 - 2x6.	2 - 2x6.	2 - 2x6.	2 - 2x6.



NOTE: 1. MIN. FRAMING MEMBERS SIZES UNLESS LARGER MEMBERS ARE SHOWN ON PLAN OR DETAILS 2. USE OSSC TABLE 2304.9.1 FOR OTHER FASTENERS REQUIRMENT NOT SHOWN 3. AT EXTERIOR HEADER SEE DET. 1/S-601



1" = 1'-0"

TYPICAL FRAMING @ DOOR & WINDOW OPNG. N.T.S.



NOTE: 1. CONNECTIONS INDICATED WITH LETTERS ARE DEFINED IN SHEAR WALL SCHEDULE 1/S-021. 2. PROVIDE F.T. LUMBER AND SHEATHING AT EXTERIOR FIRE RATED WALLS WHERE REQUIRED. SEE ARCHITECTURAL DRAWINGS.



NOTE: 1. CONNECTIONS INDICATED W/ LETTERS ARE DEFINED IN S.W. SCHED. 1/S-021

S-022

1 TYPICAL INTERIOR BEARING / SHEAR WALL

1" = 1'-0"





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1 FIRST FLOOR/FOUNDATION PLAN

1/8" = 1'-0"

	- 12" MAT SLAB W/ #5 @ 3" COVER FROM B/SLA	12" O.C. E/W TOP AND BO B, 1" COVER FROM T/SLA	DT. B 1 S-301			1 S-501 TYP.		ADDITIONAL (6) #5 TO YP. WITHIN 5 FT OF ALONG PERIMETER O
<u>2' - 6"</u>	5 12' - 6"	6 12' - 6"	163' - 10" 12' - 6"	3 12' - 6"	9 12' - 6"	10 12'-6"		6" 12
			A		A		A T	
	6 23' - 7"	1 S-022	6 23' - 7"		6 23' - 7"		6 23' - 7"	
F	F X	F 2 10' - 3"	A F X	F 2 10' - 3"		F F 2 10' - 3"	A	F 2 10' - 3"
								[2]10'-3"
	6 23' - 7"	1 S-022	6 23' - 7"		6 23' - 7"		6 23' - 7"	
	A					 EL. = 0'-0"		
	12" MAT SLAB W/ #5 @ 12 3" COVER FROM B/SLAB	2" O.C. E/W TOP AND BOT				1 S-501 TYP.		ADDITIONAL (6) #5 TYP. WITHIN 5 FT O ALONG PERIMETEF
								0. 2.
	6 23' - 7"	TYP. 1 S-022	6 23' - 7"		6 23' - 7"		¥	
(F) -	A F	(F)	A F	(F)		F (F)		F
F	F	2 10' - 3" 2 10' - 3" F	F	2 10' - 3" 2 10' - 3" F		2 10' - 3" F 2 10' - 3" F		2 10' - 3" F 2 10' - 3"
		TYP.			A		E	
	6 23' - 7"	1 S-022	6 23' - 7"		6 23' - 7"	T.O. <u>CONC.</u> EL. = 0'-0"	3 23' - 7"	
	A		A	0	A		E	
			1 S-301					





#5 TOP & BOT. T OF SLAB EDGES ER OF BUILDING



LEGEND:



SHEET NOTES:

- 1. FOR GENERAL STRUCTURAL NOTES AND ABBREVIATIONS SEE S-000 SERIES
- 2. FOR TYPICAL CONCRETE DETAILS, SEE S-01X SERIES. FOR ADDITIONAL CONCRETE DETAILS, SEE S-500 SERIES
- 3. FOR TYPICAL WOOD DETAILS, SEE S-02X SERIES. FOR ADDITIONAL WOOD DETAILS, SEE S-700 SERIES 4. VERIFY ALL DIMENSIONS, CURBS, ETC. WITH
- ARCHITECTURAL DRAWINGS PRIOR TO START OF CONSTRUCTION
- 5. REFER TO ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS FOR SLEEVES, BLOCK OUTS, AND OTHER ITEMS TO BE COORDINATED WITH STRUCTURAL DRAWINGS
- 6. SPECIFICATIONS AND DETAILING OF ALL WATERPROOFING AND DRAINAGE ITEMS ALTHOUGH INDICATED ON THE STRUCTURAL DRAWINGS FOR INFORMATION PURPOSES ONLY, ARE THE DESIGN OF WATER PROOFING & DRAINAGE **RESPONSIBILITY OF OTHERS**
- 7. AT INTERIOR NON-BEARING WALLS, ASSUME 2x4 STUDS AT 24" O.C. PROVIDE ALLOWANCE FOR DEFLECTION CLIP AT NON-BEARING WALLS 8. PROVIDE HEADERS AT EXTERIOR WALLS, INTERIOR CORRIDOR WALLS AND OVER OPENINGS IN BEARING
- WALLS, REFER TO SCHEDULE 5 / S-022





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1 SECOND FLOOR FRAMING PLAN

1/8" = 1'-0"

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LEGEND:

	STUD WALL
	STRUCTURAL WALL (B)
]	NON-STRUCTURAL WALL
	NON-STRUCTURAL WALL (B)
× 40	6x6 WOOD POST
	WD. COLUMN (B)
<u>}</u>	WD. OR STL. BEAM
HDR }	HEADER BELOW PER SCH. 5 / S-022
<u> </u>	SIMPSON STRAP, SEE SCH. 12 / S-701
SW X'-XX"	SHEAR WALL SHTG. S.W. MARK, SEE SCH. 1 / S-022 MIN. LENGTH
	OPNG.
HD	SIMP. 'HDU' HOLDOW, SEE 5 / S-501
	 CLT MAJOR AXIS SPAN DIRECTION (PANEL THICKNESS PER PLAN)
	DEPRESSION / SLAB STEP
EL. XX'-XX"	TOP OF SLAB ELEVATION WHERE 0'-0" = USGS 20.31'

SHEET NOTES:

- 1. FOR GENERAL STRUCTURAL NOTES AND ABBREVIATIONS SEE S-000 SERIES
- 2. FOR TYPICAL WOOD DETAILS, SEE S-02X SERIES. FOR ADDITIONAL WOOD DETAILS, SEE S-700 SERIES
 3. VERIFY ALL DIMENSIONS, CURBS, ETC. WITH
- 3. VERIFY ALL DIMENSIONS, CURBS, ETC. WITH ARCHITECTURAL DRAWINGS PRIOR TO START OF CONSTRUCTION
- 4. REFER TO ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS FOR SLEEVES, BLOCK OUTS, AND OTHER ITEMS TO BE COORDINATED WITH STRUCTURAL DRAWINGS
- 5. SPECIFICATIONS AND DETAILING OF ALL WATERPROOFING AND DRAINAGE ITEMS ALTHOUGH INDICATED ON THE STRUCTURAL DRAWINGS FOR INFORMATION PURPOSES ONLY, ARE THE DESIGN OF WATER PROOFING & DRAINAGE RESPONSIBILITY OF OTHERS
 6. AT INTERIOR NON-BEARING WALLS, ASSUME 2x4 STUDS AT
- AT INTERIOR NON-BEARING WALLS, ASSUME 2x4 STUDS AT 24" O.C. PROVIDE ALLOWANCE FOR DEFLECTION CLIP AT NON-BEARING WALLS
- 7. PROVIDE HEADERS AT EXTERIOR WALLS, INTERIOR CORRIDOR WALLS AND OVER OPENINGS IN BEARING WALLS, REFER TO SCHEDULE 5 / S-022





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12/21/2020

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JOB NO.	20149.10
DRAWN	CJ
DATE	4/20/2021
SCALE	1/8" = 1'-0"
FILENAME	
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1 THIRD FLOOR FRAMING PLAN S-113

1/8" = 1'-0"

LEGEND:

	STUD WALL
	STRUCTURAL WALL (B)
	NON-STRUCTURAL WALL
	NON-STRUCTURAL WALL (B)
676 10	6x6 WOOD POST
	WD. COLUMN (B)
<u>}</u>	WD. OR STL. BEAM
, HDR לל	HEADER BELOW PER SCH. 5 / S-022
<u> </u>	SIMPSON STRAP, SEE SCH. 12 / S-701
SŴ X'-XX"	SHEAR WALL SHTG. S.W. MARK, SEE SCH. 1 / S-022 MIN. LENGTH
	OPNG.
HD	SIMP. 'HDU' HOLDOW, SEE 5 / S-501
	 CLT MAJOR AXIS SPAN DIRECTION (PANEL THICKNESS PER PLAN)
	DEPRESSION / SLAB STEP
EL. XX'-XX"	TOP OF SLAB ELEVATION WHERE 0'-0" = USGS 20.31'

SHEET NOTES:

- 1. FOR GENERAL STRUCTURAL NOTES AND ABBREVIATIONS SEE S-000 SERIES
- 2. FOR TYPICAL WOOD DETAILS, SEE S-02X SERIES. FOR ADDITIONAL WOOD DETAILS, SEE S-700 SERIES
 3. VERIFY ALL DIMENSIONS, CURBS, ETC. WITH
- ARCHITECTURAL DRAWINGS PRIOR TO START OF CONSTRUCTION
- 4. REFER TO ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS FOR SLEEVES, BLOCK OUTS, AND OTHER ITEMS TO BE COORDINATED WITH STRUCTURAL
- DRAWINGS 5. SPECIFICATIONS AND DETAILING OF ALL WATERPROOFING AND DRAINAGE ITEMS ALTHOUGH INDICATED ON THE STRUCTURAL DRAWINGS FOR INFORMATION PURPOSES ONLY, ARE THE DESIGN OF WATER PROOFING & DRAINAGE
- **RESPONSIBILITY OF OTHERS** 6. AT INTERIOR NON-BEARING WALLS, ASSUME 2x4 STUDS AT 24" O.C. PROVIDE ALLOWANCE FOR DEFLECTION CLIP AT NON-BEARING WALLS 7. PROVIDE HEADERS AT EXTERIOR WALLS, INTERIOR
- CORRIDOR WALLS AND OVER OPENINGS IN BEARING WALLS, REFER TO SCHEDULE 5 / S-022

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KIND PROJECT

THE KIND PROJECT INVESTORS, LP

415 & 421 F STREET W. SACRAMENTO, CA 97605 REVISIONS

2 PLAN CHECK COMMENTS

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LEGEND:

	STUD WALL
	STRUCTURAL WALL (B)
	NON-STRUCTURAL WALL
	NON-STRUCTURAL WALL (B)
⊠ 610	6x6 WOOD POST
	WD. COLUMN (B)
\sim	WD. OR STL. BEAM
<u>, HDR</u> ,	HEADER BELOW PER SCH. 5 / S-022
<u>}</u>	SIMPSON STRAP, SEE SCH. 12 / S-70
SW X'-XX"	SHEAR WALL SHTG. S.W. MARK, SEE SCH. 1 / S-022 MIN. LENGTH
	OPNG.
HD	SIMP. 'HDU' HOLDOW, SEE 5 / S-501
	 CLT MAJOR AXIS SPAN DIRECTION (PANEL THICKNESS PER PLAN)
7777) 17777	DEPRESSION / SLAB STEP
EL. XX'-XX"	TOP OF SLAB ELEVATION WHERE 0'-0" = USGS 20.31'

SHEET NOTES:

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LEGEND:

	STUD WALL
	STRUCTURAL WALL (B)
	NON-STRUCTURAL WALL
	NON-STRUCTURAL WALL (B)
N CTO	6x6 WOOD POST
	WD. COLUMN (B)
<u>}</u>	WD. OR STL. BEAM
_ HDR	HEADER BELOW PER SCH. 5 / S-022
<i>⊱</i> }	SIMPSON STRAP, SEE SCH. 12 / S-701
SW X'-XX"	SHEAR WALL SHTG. S.W. MARK, SEE SCH. 1 / S-022 MIN. LENGTH
	OPNG.
HD	SIMP. 'HDU' HOLDOW, SEE 5 / S-501
	 CLT MAJOR AXIS SPAN DIRECTION (PANEL THICKNESS PER PLAN)
	7 DEPRESSION / SLAB STEP
EL. XX'-XX"	TOP OF SLAB ELEVATION

SHEET NOTES:

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LEGEND:

	STUD WALL
	STRUCTURAL WALL (B)
	NON-STRUCTURAL WALL
	NON-STRUCTURAL WALL (B)
⊠ 670	6x6 WOOD POST
	WD. COLUMN (B)
<u>}</u>	WD. OR STL. BEAM
<mark>, HDR</mark> ,	HEADER BELOW PER SCH. 5 / S-022
<u>}</u>	SIMPSON STRAP, SEE SCH. 12 / S-701
SW X'-XX"	SHEAR WALL SHTG. S.W. MARK, SEE SCH. 1 / S-022 MIN. LENGTH
	OPNG.
• HD	SIMP. 'HDU' HOLDOW, SEE 5 / S-501
	 CLT MAJOR AXIS SPAN DIRECTION (PANEL THICKNESS PER PLAN)
	DEPRESSION / SLAB STEP
EL. XX'-XX"	TOP OF SLAB ELEVATION WHERE 0'-0" = USGS 20.31'

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WALLS, REFER TO SCHEDULE 5 / S-022

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415 & 421 F STREET W. SACRAMENTO, CA 97605 REVISIONS 2 PLAN CHECK COMMENTS 12/21/2020

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LEGEND:

	STUD WALL
	STRUCTURAL WALL (B)
	NON-STRUCTURAL WALL
	NON-STRUCTURAL WALL (B)
× 610	6x6 WOOD POST
	WD. COLUMN (B)
<u>}</u>	WD. OR STL. BEAM
	HEADER BELOW PER SCH. 5 / S-022
<u>}</u>	SIMPSON STRAP, SEE SCH. 12 / S-701
SW X'-XX"	SHEAR WALL SHTG. S.W. MARK, SEE SCH. 1 / S-022 MIN. LENGTH
	OPNG.
HD	SIMP. 'HDU' HOLDOW, SEE 5 / S-501
	 CLT MAJOR AXIS SPAN DIRECTION (PANEL THICKNESS PER PLAN)
	7 DEPRESSION / SLAB STEP
EL. XX'-XX"	TOP OF SLAB ELEVATION WHERE 0'-0" = USGS 20.31'

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ngs and written material appearing nerein constitute original and unpublished work of the structural Engineer and may not be duplicated, used or disclosed without consent of str S-125 ROOF CLT PANEL PLAN 5/10/2021 10:46:24 AM C.(Users\leandro.pimenta\Documents\Revit20149 10-The Kind Project-S20-Central lean

1 ROOF - CLT PANEL PLAN S-125

1/8" = 1'-0"

|

LEGEND:

	STUD WALL	
	□ STRUCTURAL WALL (B)	
	NON-STRUCTURAL WALL	
	NON-STRUCTURAL WALL (B)	
M CAN	6x6 WOOD POST	
	WD. COLUMN (B)	
<u> </u>	WD. OR STL. BEAM	
<mark>HDR</mark>	HEADER BELOW PER SCH. 5 / S-022	
<u>}</u>	SIMPSON STRAP, SEE SCH. 12 / S-701	
SW X'-XX"	SHEAR WALL SHTG. S.W. MARK, SEE SCH. 1 / S-022 MIN. LENGTH	
	OPNG.	
HD	SIMP. 'HDU' HOLDOW, SEE 5 / S-501	
-	 CLT MAJOR AXIS SPAN DIRECTION (PANEL THICKNESS PER PLAN) 	
	7 DEPRESSION / SLAB STEP	
EL. XX'-XX"	TOP OF SLAB ELEVATION WHERE 0'-0" = USGS 20.31'	

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2 S-301

4 MEZZANINE FLOOR - PARTIAL PLAN S-301

1/4" = 1'-0"

3 S-301

SHEAR WALL AT CORRIDOR

1/4" = 1'-0"

TRANSVERSE BUILDING SECTION S-301

1/8" = 1'-0"

+ paddon	+ PLANNERS	1715 R Street., Ste. 200 Sacramento, CA 95811 wp-architects.com
williams	ARCHITECTS	2237 Douglas Blvd., Ste. 160 Roseville , CA 95661 916.786.8178

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BUILDING SECTIONS AND ENLARGED PLANS

ANCHOR

Ø

5/8"

5/8"

5/8"

7/8"

1"

1"

HOLDOWN B/S OF

POST PER PLAN STAGGER AS SHOWN

MARK HOLDOWN

А

В

С

D

Е

F

HDU2

HDU4

HDU5

HDU8

HDU11

HDU14

NOTES: 1. POST SIZE INDICATED ON PLAN SHALL BE USED UNLESS IT IS SMALLER THAN MIN. POST SIZE INDICATED IN SCHEDULE. ALL HARDWARE (ALL THREAD ROD, COUPLERS, WASHERS, NUTS) IN CONTACT WITH CONCRETE TO BE HOT DIPPED GALVANIZED.

5 HOLDOWN SCHEDULE

NOTE: FOR INFORMATION NOT NOTED SEE DETAIL 1 / S-501

WASHER SIZE	CAPACITY (LBS.)	MIN. POST SIZE
3x3x3/8"	3075	4x4
3x3x3/8"	4565	4x4
3x3x1/2"	5645	4x4
3x3x1/2"	7870	4x6
3x3x1/2"	11175	4x8
3x3x1/2"	14375	4x8 OR 6x6

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- 2 PLAN CHECK COMMENTS 3 PLAN CHECK
- 3 PLAN CHECKCOMMENTS4 SSK-3

12/21/2020 2/24/2021 4/20/2021

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